32-34 Georges Bay Esplanade St Helens Tasmania 7216 T: 03 6376 7900 ABN 96 017 131 248



Development Applications

Notice is hereby given under Section 57(3) of the Land Use Planning & Approvals Act 1993 that an application has been made to the Break O' Day Council for a permit for the use or development of land as follows:

DA Number DA 2025 / 00046

Applicant J. Binns

Proposal Residential – Construction of a Dwelling and Retrospective Approval of Outbuilding

Structures

Location 60 Treloggen Drive, Binalong Bay

Plans and documents can be inspected at the Council Office by appointment, 32 - 34 Georges Bay Esplanade, St Helens during normal office hours or online at www.bodc.tas.gov.au.

Representations must be submitted in writing to the General Manager, Break O'Day Council, 32 -34 Georges Bay Esplanade, St Helens 7216 or emailed to admin@bodc.tas.gov.au, and referenced with the Application Number in accordance with section 57(5) of the abovementioned Act during the fourteen (14) day advertised period commencing on Wednesday 20th August 2025 until 5pm Tuesday 2nd September 2025.

John Brown GENERAL MANAGER

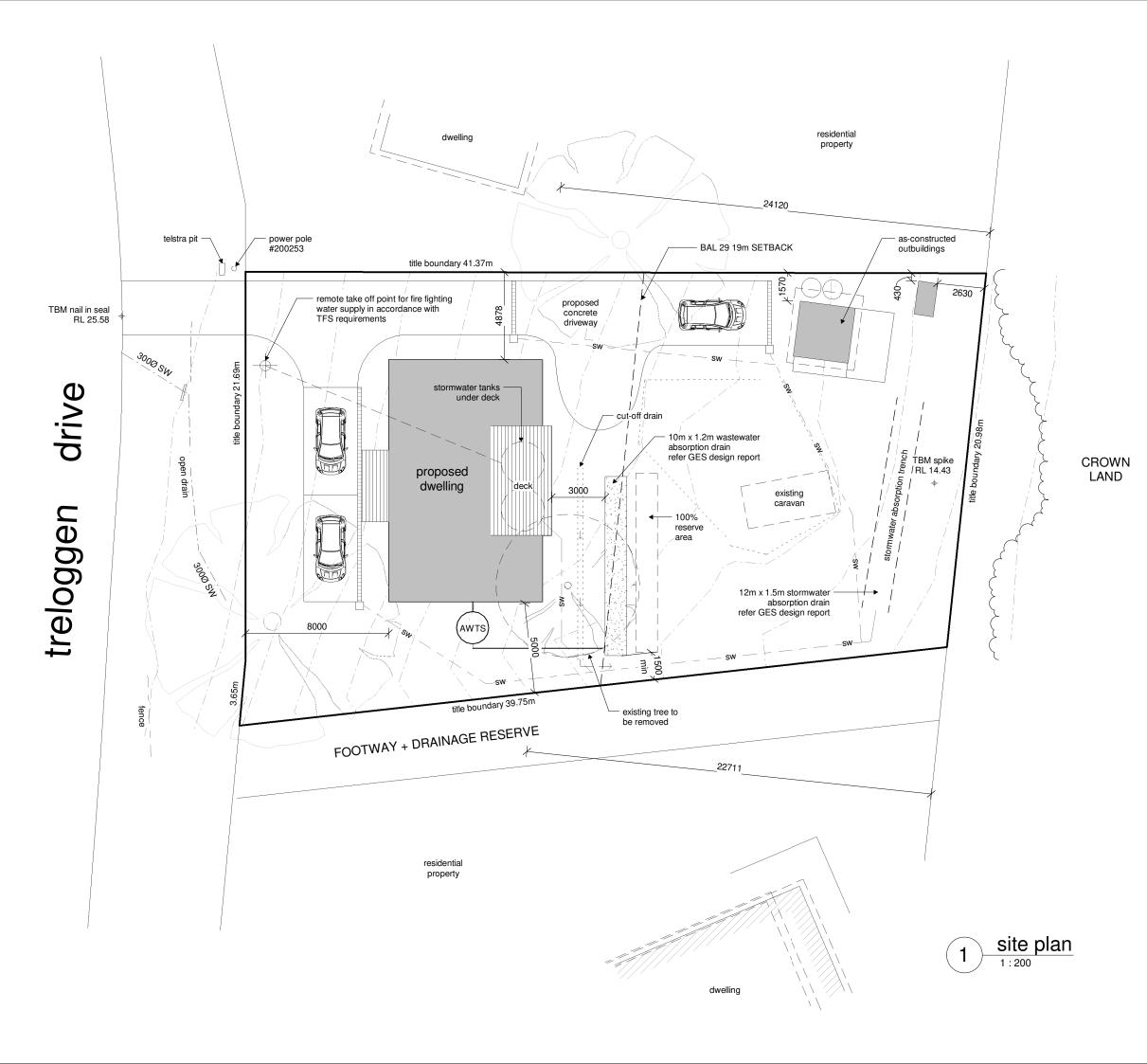
proposed dwelling + as-constucted outbuilding

katie ford 60 treloggen drive binalong bay tasmania 7216

planning application

Building Areas

| dwelling area | 98.75 |
|---------------|--------|
| deck 2 | 20.99 |
| deck 2 | 6.24 |
| | 125 00 |





| 1 | Revision 1 | Date 1 |
|------|--------------|--------|
| REV: | DESCRIPTION: | DATE: |
| | | |

PROJEC

proposed dwelling + outbuilding k. ford 60 treloggen drive binalong bay TAS 7216

DRAWING TITLE:

site plan

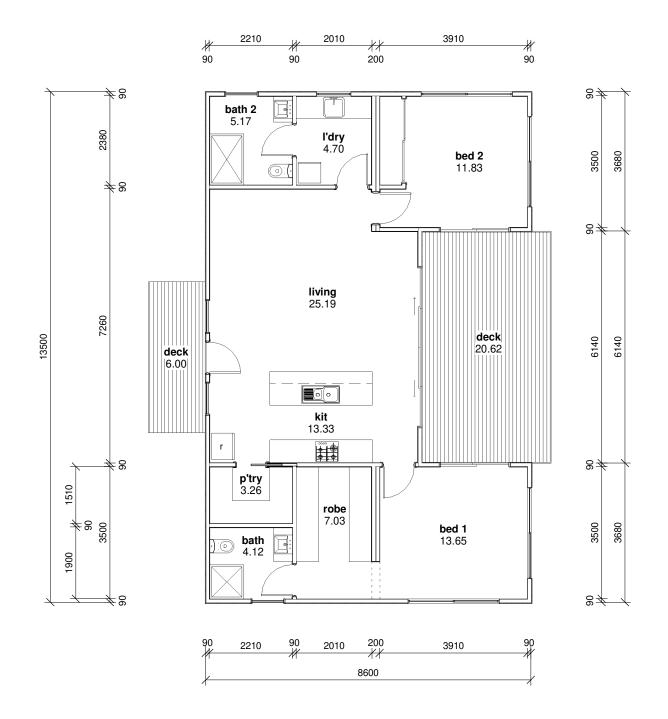
| DRAWING NO: | DRAWN BY: JB |
|--------------|-----------------|
| a04 | DATE: 20.05.25 |
| SCALE: 1:200 | PROJECT: 1123FO |



www.jenniferbinnsdesign.com.au 0439 765 452 : mail @ jenniferbinnsdesign.com.au 52 cecilia street st helens tasmania 7216



ACCREDITATION NO: CC 1269L



proposed floor plan

1:100



REV: DESCRIPTION: DATE:

PROJEC

proposed dwelling + outbuilding k. ford 60 treloggen drive binalong bay TAS 7216

DRAWING TITLE:

proposed floor plan

DRAWING NO: DRAWN BY: JB

DATE: 20.05.25

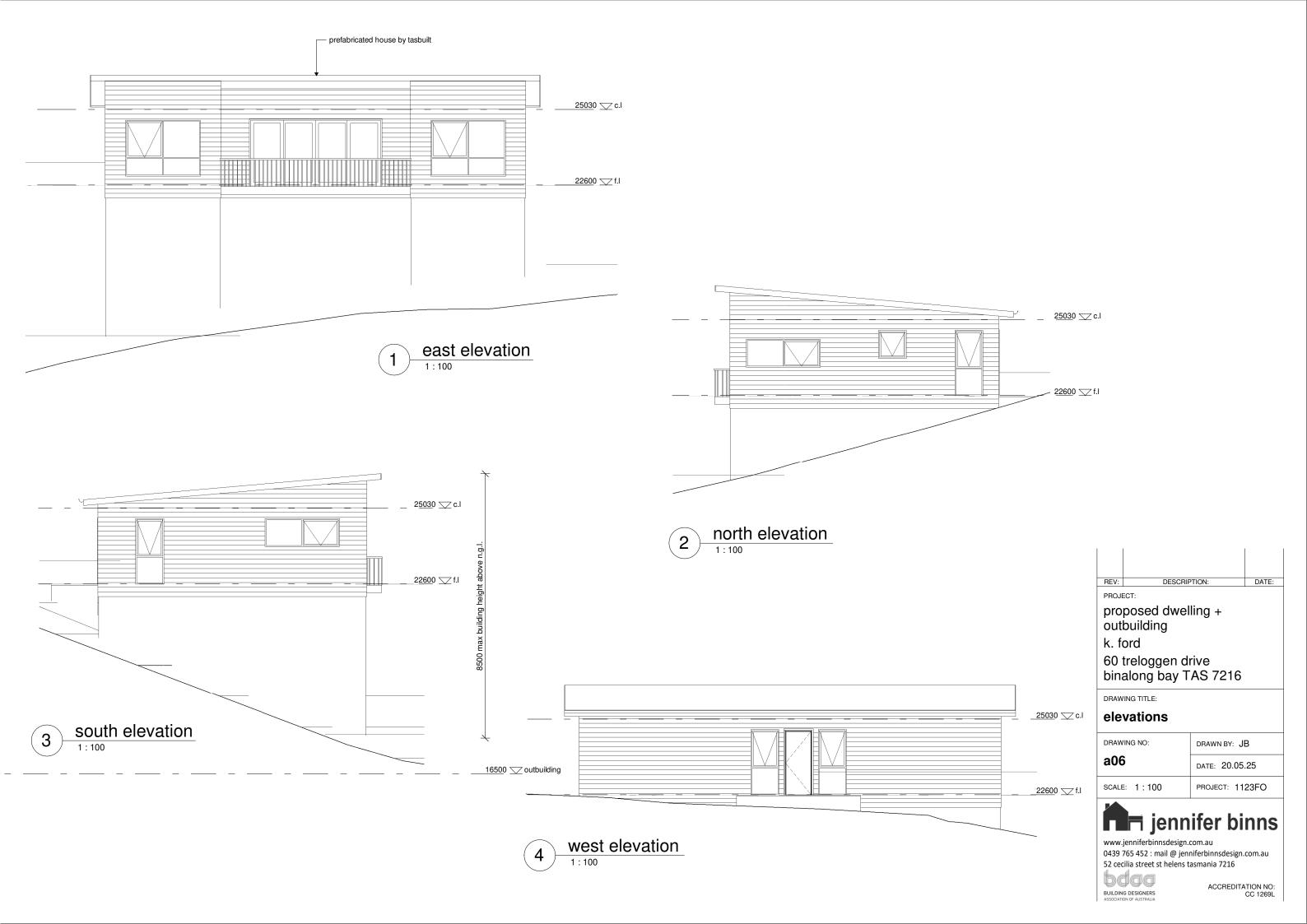
SCALE: 1:100 PROJECT: 1123FO

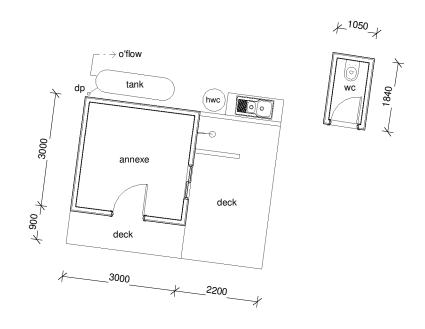


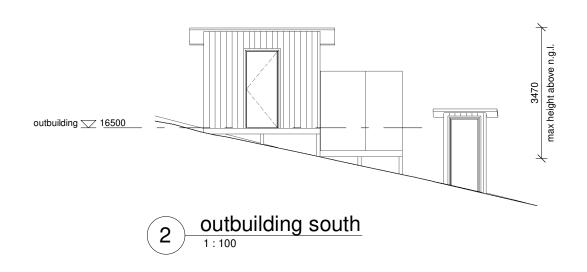
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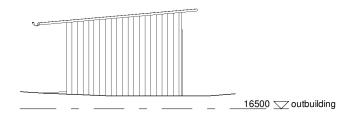


ACCREDITATION NO: CC 1269L



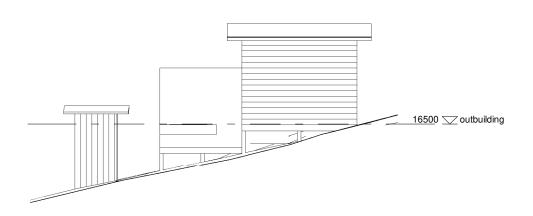




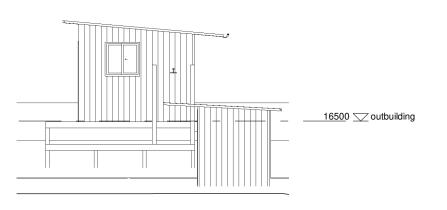


3 outbuilding west

1 as-constructed outbuilding



4 outbuilding north



5 outbuilding east



REV: DESCRIPTION: DATE:

PROJEC

proposed dwelling + outbuilding k. ford 60 treloggen drive binalong bay TAS 7216

DRAWING TITLE:

as-constructed outbuilding

| DRAWING NO: | | DRAWN BY: JB | |
|-------------|--------------|-----------------|--|
| | | DATE: 20.05.25 | |
| | SCALE: 1:100 | PROJECT: 1123FO | |



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BUILDING DESIGNERS
ASSOCIATION OF AUSTRALIA

ACCREDITATION NO: CC 1269L

proposed dwelling + as-constructed outbuilding

katie ford 60 treloggen drive binalong bay tasmania 7216

planning compliance report

may 20 2025

jennifer binns building design

52 cecilia street st helens tasmania 7216 mail@jenniferbinnsdesign.com.au: 0439 765 452

Introduction

This report aims to demonstrate compliance with relevant planning standards for a proposed dwelling and as-constructed outbuilding for Katie Ford at 60 Treloggen Drive Binalong Bay (c.t.104144/1). The report aims to take into consideration the intent, values and objectives of the Tasmanian Planning Scheme and address all scheme standards applicable to this development.

The proposed development relies on **Performance Criteria** to satisfy relevant planning standards and this application is to be read in conjunction with drawings and reports submitted for the development.

Development Site Details

The development site is a residential allotment within the Binalong Bay township. The property is considered bushfire prone but has previously been cleared of vegetation and no further native vegetation removal is proposed as part of this application. The dwelling will be serviced by on-site wastewater and stormwater management systems and the existing vehicle access and driveway will be upgraded.

Zone: Low Density Residential



Development Details

The proposed development is a high set prefabricated dwelling. This application includes retrospective approval for the as-constructed outbuildings on site.

Use Class: Residential + Visitor Accommodation

Applicable Planning Codes

The proposed development is in the *Residential* and *Visitor Accommodation* use classes which in the *Low Density Residential* Zone are *No Permit Required* and *Permitted* uses.

The following zone standards and codes of the Tasmanian Planning Scheme are applicable to the proposed development:

- Zone 10.0 LOW DENSITY RESIDENTIAL ZONE
- Code 2.0 PARKING AND SUSTAINABLE TRASNPORT CODE

Table 10.3 LOW DENSITY RESIDENTIAL USE STANDARDS

10.3.1 Discretionary uses

A1 Not Applicable

The proposed development is not a discretionary use.

A2 Not Applicable

The proposed development is not a discretionary use.

A3 Not Applicable

The proposed development is not a commercial use.

A4 Not Applicable

The proposed development is not a discretionary use.

10.3.2 Visitor accommodation

P1 Performance Solution

The proposed dwelling will be utilised for visitor accommodation for part of the year. The development is a new building and the dwelling has a floor area of 100m². The development is a single dwelling and is not expected to generate noise beyond standard residential use. The use of the dwelling for visitor accommodation is in keeping with the character of Binalong Bay.

A2 Not Applicable

No strata lots are proposed.

Table 10.4 LOW DENSITY RESIDENTIAL DEVELOPMENT STANDARDS

10.4.1. Residential density for multiple dwellings

A1 Not Applicable

The proposed development does not include multiple dwellings.

10.4.2 Building height

A1 Acceptable Solution

The proposed dwelling is a prefabricated building on a sloping site and has a maximum height of 8.5m above natural ground level.

10.4.3 Setback

A1 Acceptable Solution

The proposed dwelling has a front setback of 8m.

P2 Performance Solution

The proposed dwelling has a minimum northern boundary setback of 4.88m. The proposed setback is only marginally less than the prescribed minimum 5m setback and does not reduce privacy or solar access to the adjoining properties. The as-constructed outbuildings have a minimum northern boundary setback of 430mm and 1570mm and a minimum rear boundary setback of 2.63m. The outbuildings are sited at the rear of the property and are small structures which do not impact on the solar access or privacy amenity of the adjoining residential properties. The siting of these small outbuildings in the rear corner of the property is considered in keeping with the residential pattern of development in the area.

10.4.4 Site coverage

A1 Acceptable Solution

The proposed development does not exceed 30% of the site area.

10.4.5 Fencing

A1 Not Applicable

No front fencing is proposed as part of this application.

Table 10.5 VILLAGE DEVELOPMENT STANDARDS FOR SUBDIVISION

Not applicable

No subdivision of land is proposed

Table C2.5 CAR PARKING USE STANDARDS

C2.5.1 Car parking numbers

A1 Acceptable Solution

The layout of the development site facilitates parking for a minimum of 2 vehicles for the proposed two bedroom dwelling in accordance with the requirements of Table 2.1.

C2.5.2 Bicycle parking numbers

Not Applicable

The proposed development does not require the provision of bicycle parking.

C2.5.3 Motorcycle parking numbers

Not Applicable

The proposed development does not require the provision of motorcycle parking.

C2.5.4 Loading bays

Not Applicable

The proposed development does not require provision of a loading bay.

C2.5.5 Number of car parking spaces within the General Residential zone and Inner Residential zone

A1 Not Applicable

The proposed development is in the Low Density Residential zone.

Table C2.6 CAR PARKING DEVELOPMENT STANDARDS

C2.6.1 Construction of parking areas

A1 Acceptable Solution

The proposed driveway and parking areas will be concrete and drained to the on-site stormwater drain.

C2.6.2 Design and layout of parking areas

A1 Acceptable Solution

The layout of the proposed parking spaces meets the prescribed requirements and the layout of the development site facilitates forward egress.

A1.2 Not Applicable

No accessible parking is required for the proposed development.

C2.6.3 Number of accesses for vehicles

A1 Acceptable Solution

The proposed development has one access point only.

A2 Not Applicable

The development site is in the Low Density Residential zone.

C2.6.4 Lighting of parking areas within the Gen. Business zone and Central Business zone

A1 Not Applicable

The development site is in the *Low Density Residential* zone.

C2.6.5 Pedestrian Access

A1.1 Not Applicable

The proposed development does not require the provision of pedestrian access paths.

A1.2 Acceptable Solution

The proposed development does not require the provision of accessible parking.

C2.6.6 Loading bays

A1 Not Applicable

The proposed development does not require the provision of a loading bay.

A2 Not Applicable

There are no commercial vehicles associated with the proposed development.

C2.6.7 Bicycle parking and storage facilities within the Gen. Business zone and Central Business zone

A1 Not Applicable

The proposed development does not require the provision of bicycle parking.

A2 Not Applicable

The proposed development does not require the provision of bicycle parking.

C2.6.8 Siting of parking and turning areas

A1 Not Applicable

The development site is in the Low Density Residential zone.

A2 Not Applicable

The development site is in the Low Density Residential zone.

Table C2.7 PARKING PRECINCT PLAN

C2.7.1 Construction of parking areas

A1 Not Applicable

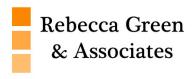
The development site is not within a parking precinct plan.



Bushfire Hazard Assessment Report & Bushfire Hazard Management Plan

60 Treloggen Drive, Binalong Bay





Prepared for (Client)

Katie Ford

7 Middle Head Road

MOSMAN NSW 2088

Assessed & Prepared by

Rebecca Green

Senior Planning Consultant & Accredited Bushfire Hazard Assessor

Rebecca Green & Associates

PO Box 2108 LAUNCESTON TAS 7250

Mobile: 0409 284 422

Version 1

30 January 2025

Job No: RGA-B2432



Executive Summary

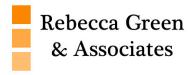
The proposed development at 60 Treloggen Drive, Binalong Bay is subject to bushfire threat. A bushfire attack under extreme fire weather conditions is likely to subject buildings at this site to considerable radiant heat, ember attack along with wind and smoke.

The site requires bushfire protection measures to protect the buildings and people that may be on site during a bushfire.

These measures include provision of hazard management areas in close proximity to the buildings, implementation of safe egress routes, establishment of a water supply and construction of buildings as described in AS 3959-2018 Construction of Buildings in Bushfire Prone Areas.

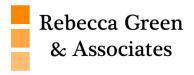
Primary responsibilities identified within this report:

| Occuries | - Catablish and maintain Harand |
|----------|--|
| Occupier | <u>Establish and maintain</u> Hazard |
| | Management Areas as described in this |
| | report, including egress and access |
| | routes. |
| | <u>Establish and maintain</u> adequate |
| | turning facilities for emergency |
| | vehicles, as described in this report. |
| | <u>Establish and maintain</u> an independent |
| | water supply for fire fighting purposes, |
| | including connection point within 3m of |
| | hardstand and signage. |
| | Design and Construct single dwelling to |
| | meet <u>BAL 29</u> (AS3959-2018). |



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Schedule 1 – Bushfire Report

1.0 Introduction

The Bushfire Attack Level (BAL) Report and Bushfire Hazard Management Plan (BHMP) has been prepared for submission with a Building Permit Application under the *Building Act 2016 & Regulations 2016*.

The Bushfire Attack Level (BAL) is established taking into account the type and density of vegetation within 100 metres of the proposed building site and the slope of the land; using the simplified method in AS 3959-2018 Construction of Buildings in Bushfire Prone Areas; and includes:

- The type and density of vegetation on the site,
- Relationship of that vegetation to the slope and topography of the land,
- Orientation and predominant fire risk,
- Other features attributing to bushfire risk.

On completion of assessment, a Bushfire Attack Level (BAL) is established which has a direct reference to the construction methods and techniques to be undertaken on the buildings and for the preparation of a Bushfire Hazard Management Plan (BHMP).

1.1 Scope

This report was commissioned to identify the Bushfire Attack Level for the existing property. ALL comment, advice and fire suppression measures are in relation to compliance with the Building Code of Australia and Australian Standards, AS 3959-2018, Construction of buildings in bushfire-prone areas.

1.2 Limitations

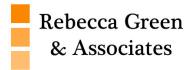
The inspection has been undertaken and report provided on the understanding that:-

- 1. The report only deals with the potential bushfire risk, all other statutory assessments are outside the scope of this report.
- 2. The report only identifies the size, volume and status of vegetation at the time the site inspection was undertaken and cannot be relied upon for any future development.
- 3. Impacts of future development and vegetation growth have not been considered.

No action or reliance is to be placed on this report; other than for which it was commissioned.

1.3 Proposal

The proposal is for the construction of a new single dwelling.



2.0 Site Description for Proposal (Bushfire Context)

2.1 Locality Plan

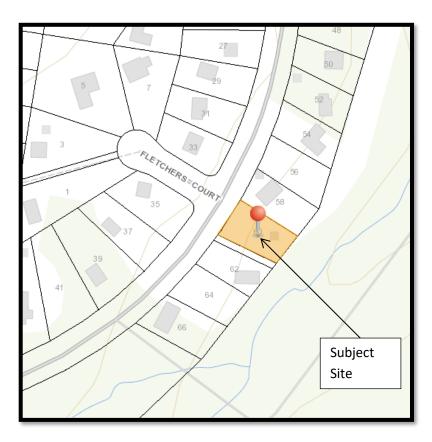
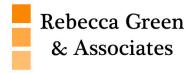


Figure 1: Location Plan of 60 Treloggen Drive, Binalong Bay

2.2 Site Details

| Property Address 60 Treloggen Drive, Binalong Bay | | |
|---|---|--|
| Certificate of Title | Volume 104144 Folio 1 | |
| Owner | Katie Ann Ford | |
| Existing Use | Vacant | |
| Type of Proposed Building Work | Construction of single dwelling | |
| BCA Classification | Dwelling – Class 1a | |
| Water Supply | On-site supply for fire fighting purposes | |
| Road Access Street Frontage – Treloggen Drive | | |

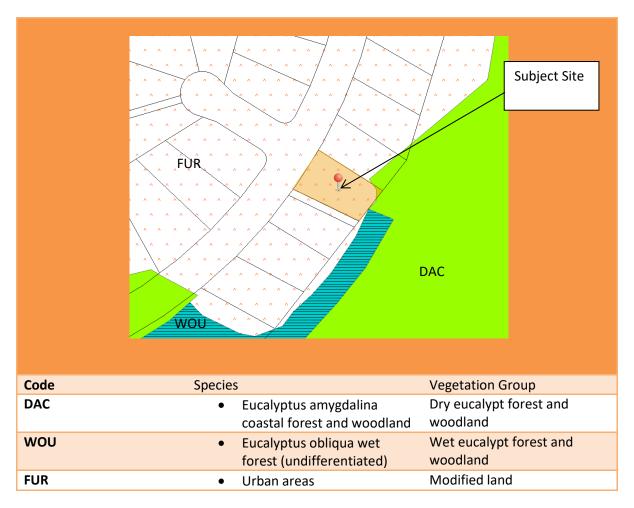


3.0 Bushfire Site Assessment

3.1 Vegetation Analysis

3.1.1 TasVeg Classification

Reference to Tasmanian Vegetation Monitoring & Mapping Program (TASVEG) indicates the land in and around the property is generally comprising of varying vegetation types including:



3.1.2 Site & Vegetation Photos





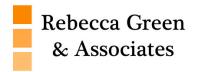
Looking southwest



Looking northwest



Looking further to northwest



3.2 BAL Assessment – Dwelling

| vegetation classification AS3959 | North ⊔ North-East ⊠ | South □ South-West ☒ | East ⊔ South-East ⊠ | West ⊔ North-West ⊠ |
|--|---------------------------------|---|---|---------------------------------|
| Group A | ☐ Forest | | | ☐ Forest |
| Group B | ☐ Woodland | ☐ Woodland | ☐ Woodland | ☐ Woodland |
| Group C | ☐ Shrub-land | ☐ Shrub-land | ☐ Shrub-land | ☐ Shrub-land |
| Group D | ☐ Scrub | ☐ Scrub | ☐ Scrub | ☐ Scrub |
| Group E | ☐ Mallee-Mulga | ☐ Mallee-Mulga | ☐ Mallee-Mulga | ☐ Mallee-Mulga |
| Group F | ☐ Rainforest | ☐ Rainforest | ☐ Rainforest | ☐ Rainforest |
| Group G | ☐ Grassland | □ Grassland | ☐ Grassland | ☐ Grassland |
| | | | | |
| | | | | |
| | | | | |
| Effective | ☑ Up/0 ⁰ | ☑ Up/0 ⁰ | ☐ Up/0 ⁰ | ☑ Up/0 ⁰ |
| slope (degrees) | □ >0-5° | □ >0-5° | ⊠ >0-5° | □ >0-5° |
| (degrees) | □ >5-10 ⁰ | □ >5-10 ⁰ | □ >5-10 ⁰ | □ >5-10 ⁰ |
| | □ >10-15 ⁰ | □ >10-15 ⁰ | □ >10-15 ⁰ | □ >10-15 ⁰ |
| | □ >15-20° | □ >15-20° | □ >15-20 ⁰ | □ >15-20° |
| Distance to | N.A. et u.e. e | N.4 | N.A. et u.e. e | N.A. a.k.u.a.a. |
| Distance to classified vegetation | Metres >100m managed/low threat | Metres 0-approx. 77m managed/low threat 77-<90m grassland >90m forest | Metres 0-<22.7m managed/low threat (subject site) 22.7-<24.7m grassland >24.7m forest | Metres >100m managed/low threat |
| | | | | |
| Likely direction of bushfire attack | | | | |
| | | | | |
| Prevailing winds | | | | |
| | | | | |
| Exclusions | a b c d <u>e</u> <u>f</u> | a b c d <u>e</u> <u>f</u> | a b c d e <u>f</u> | a b c d <u>e</u> <u>f</u> |
| | | | | 5 5 |
| BAL Value (FDI 50) | BAL – LOW | BAL – 12.5 | BAL – 29 | BAL – LOW |



The Bushfire Attack Level shall be classified BAL-LOW where the vegetation is one or a combination of any of the following:

- (a) Vegetation of any type that is more than 100 metres from the site.
- (b) Single areas of vegetation less than 1 hectare in area and not within 100m of other areas of vegetation being classified.
- (c) Multiple areas of vegetation less than 0.25 hectare in area and not within 20 metres of the site, or each other.
- (d) Strips of vegetation less than 20 metres in width (measured perpendicular to the elevation exposed to the strip of vegetation) regardless of length and not within 20 metres of the site or each other, or other areas of vegetation being classified.
- (e) Non-vegetated areas, including waterways, roads, footpaths, buildings and rocky outcrops.
- (f) Low threat vegetation, including grassland managed in a minimal fuel condition, maintained lawns, golf courses, maintained public reserves and parklands, vineyards, orchards, cultivated gardens, commercial nurseries, nature strips and windbreaks.

NOTE: Minimal fuel condition means there is insufficient fuel available to significantly increase the severity of the bushfire attack (recognisable as short-cropped grass for example, to a nominal height of 100mm).

3.2 Specified Hazard Management Areas

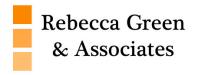
Hazard management areas are to be established <u>and maintained</u> between the bushfire prone vegetation and the building at a distance equal to, or greater than the separation distance specified for the Bushfire Attack Levels (BAL) in table 2.6 of *Australian Standard 3959-2018 Construction of Buildings in Bushfire Prone Areas*.

Where the Hazard Management Areas can be increased around the building and the classified vegetation in accordance with table 2.6 of Australian Standard 3959, the risk from bushfire attack can reduce.

Single Dwelling

| Distance from Predominant vegetation for BAL 29 | North/ North-East | South/ South-West | East/ South-East | West/ North-West |
|---|-----------------------------|----------------------|---------------------------------|---------------------|
| | To title boundary | To title boundary | To title boundary (min. 19m) | To title boundary |
| | Metres | Metres | Metres | Metres |

The separation distance for the SPECIFIED Hazard Management Area is to be shown on the attached Bushfire Hazard Management Plan measured from the external walls (Façade) of the building in metres along the ground to the bushfire hazard vegetation (if applicable).



3.3 Outbuildings

Not applicable, existing.

3.4 Road Access

Roads are to be constructed to provide vehicle access to the site to assist firefighting and emergency personnel to defend the building or evacuate occupants; and provide access at all times to the water supply for firefighting purposes on the building site.

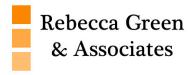
Private access roads are to be upgraded/maintained from the entrance to the property cross over with the public road through to the dwelling. Private access roads are to be designed, constructed and maintained to a standard not less than Table 2.

| New / Existing Road Access and Driveways | Private access driveway / roads are to be constructed/maintained from the entrance of the property cross over at the public road (Treloggen Drive) through to the buildings and |
|--|---|
| | on-site dedicated fire fighting water. Private access roads are to be maintained to a standard not less than specified in Table 2 B. |
| | Note: Slope of access is approximately 15 degrees, therefore access to be sealed. |

Table 2: Requirements for Property Access

The following design and construction requirements apply to property access length is 30 metres or greater or access for a fire appliance to a fire fighting water point:

- (i) All weather construction;
- (ii) Load capacity of at least 20 tonnes, including for bridges and culverts;
- (iii) Minimum carriageway width of 4 metres;
- (iv) Minimum vertical clearance of 4 metres;
- (v) Minimum horizontal clearance of 0.5 metres from the edge of the carriageway;
- (vi) Cross falls of less than 3 degrees (1:20 or 5%);
- (vii) Dips less than 7 degrees (1:8 or 12.5%) entry and exit angle;
- (viii) Curves with a minimum inner radius of 10 metres;
- (ix) Maximum gradient of 15 degrees (1:3.5 or 28%) for sealed roads, and 10 degrees (1:5.5 or 18%) for unsealed roads; and
- (x) Terminate with a turning area for fire appliances provided by one of the following:
 - a) A turning circle with a minimum inner radius of 10 metres;
 - b) A property access encircling the building; or
 - c) A hammerhead "T" or "Y" turning head 4 metres wide and 8 metres long.



3.5 Water Supply

A building that is constructed in a designated bushfire prone area must provide access at all times to a sufficient supply of water for firefighting purposes on the building site.

The exterior elements of a habitable building in a designated Bushfire prone area must be within reach of a 120m long hose (reticulated) or 90m long hose (static) (lay) connected to –

- (i) A fire hydrant system designed and constructed in accordance with TasWater Supplement to Water Supply Code of Australia WSA 03-2011-3.1 MRWA Edition 2.0; or
- (ii) A stored water supply in a water tank, swimming pool, dam or lake available for fire fighting at all times which has the capacity of at least 10,000L for each separate building area to be protected.

| New | On-site water supply is to be established and |
|--|--|
| On-site Dedicated Fire Fighting Water Supply | maintained, no fire hydrant was sited during site |
| | inspection within 120m of the furthest part of |
| | the dwelling, with connection point less than 3m |
| | from hardstand. |
| | |
| | A <u>water tank</u> of at least 10,000 litres per building |
| | area to be protected and above ground pipes |
| | and fittings used for a stored water supply must |
| | be of non-rusting, non-combustible, non-heat- |
| | deforming materials and must be situated more |
| | than 6m from a building area to be protected. |

Table 3B: Requirements for Static Water Supply for Fire Fighting

| Column 1 | | Column 2 | |
|----------|---|--|--|
| Element | | Requirement | |
| A. | Distance between building area to be protected and water supply | The following requirements apply: (1) The building area to be protected must be located within 90 metres of the fire fighting water point of a static water supply; and (2) The distance must be measured as a hose lay, between the fire fighting water point and the furthest part of the building area. | |
| B. | Static Water Supplies | A static water supply: May have a remotely located offtake connected to the static water supply; May be a supply for combined use (fire fighting and other uses) but the specified minimum quantity of fire fighting water must be available at all times; Must be a minimum of 10,000 litres per building area to be protected. This volume of water must not be used for any other purpose including fire fighting sprinkler or spray systems; | |



| C. | Fittings, pipework and accessories (including stands and tank | (4) Must be metal, concrete or lagged by noncombustible materials if above ground; and (5) If a tank can be located so it is shielded in all directions in compliance with Section 3.5 of AS 3959-2018 the tank may be constructed of any material provided that the lowest 400mm of the tank exterior is protected by: (a) Metal; (b) Non-combustible material; or (c) Fibre-cement a minimum 6mm thickness. Fittings and pipework associated with a fire fighting water point for a static water supply must: (a) Have a minimum nominal internal diameter of |
|----|---|---|
| | supports) | (b) Be fitted with a valve with a minimum nominal diameter of 50mm; (c) Be metal or lagged by non-combustible materials if above ground; (d) Where buried, have a minimum depth of 300mm; (e) Provide a DIN or NEN standard forged Storz 65mm coupling fitted with a suction washer for connection to fire fighting equipment; (f) Ensure the coupling is accessible and available for connection at all times; (g) Ensure the coupling is fitted with a blank cap and securing chain (minimum 220mm length); (h) Ensure underground tanks have either an opening at the top of not less than 250mm diameter or a coupling compliant with this Table; and (i) Where a remote offtake is installed, ensure the offtake is in a position that is: (a) Visible; (b) Accessible to allow connection by fire fighting equipment; (c) At a working height of 450-600mm above ground level; and (d) Protected from possible damage, including damage from vehicles. |
| D. | Signage for static water connections | The fire fighting water point for a static supply must be identified by a sign permanently fixed to the exterior of the assembly in a visible location. The sign must: (a) Comply with water tank signage requirements within AS 2304:2019; or (b) Comply with the Tasmania Fire Service Water Supply Signage Guideline published by the Tasmania Fire Service. |
| E. | Hardstand | A hardstand area for fire appliances must be provided: (a) No more than three metres from the fire fighting water point, measured as a hose lay (including the minimum water level in dams, swimming pools and the like); |



- (b) No closer than six metres from the building area to be protected;
- (c) With a minimum width of three metres constructed to the same standard as the carriageway; and
- (d) Connected to the property access by a carriageway equivalent to the standard of the property access.

4.0 Layout Options

Not relevant to this proposal.

5.0 Other Planning Provisions

Not relevant to this proposal.

6.0 Conclusions and Recommendations

Mitigation from bushfire is dependent on the careful management of the site by maintaining reduced fuel loads within the hazard management areas and within the site.

The site has been assessed as requiring buildings (Single Dwelling) to conform to or exceed BAL 29 requirements based on AS 3959 – 2018 Construction of Buildings in Bushfire Prone Areas.

Access

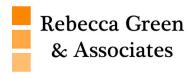
The driveway is to be <u>constructed/maintained</u> to meet Table 2B. Requirements for Property Access, Director's Determination – Bushfire Hazard Areas, Version 1.2.

Water Supplies

Water supply tank at least 10,000 litres (10,000l per building area to be protected) is to be established and maintained, with a fitting suitable for TFS access, meeting the requirements for Reticulated Water Supply for Fire Fighting, Table 3B, Director's Determination — Bushfire Hazard Areas, Version 1.2.

Fuel Managed Areas

Hazard Management Areas as detailed within the plan shall be constructed and maintained as detailed in Section 2 of Schedule 2 (where applicable).



Schedule 2 – Bushfire Hazard Management Plan

1.0 Introduction

The Bushfire Hazard Management Plan (BHMP) is developed from the results of a Bushfire Attack Level (BAL) Assessment Report prepared for the site in accordance with Australian Standard 3959. The BHMP provides reference and information to existing and subsequent owners on their responsibilities for the establishment, maintenance and future management of their property to reduce the risk of bushfire attack and includes: -

- Establishment of a Hazard Management Area in and around the existing and/or proposed buildings,
- Specifications of Private access road construction,
- Provision on firefighting water supply,
- Construction requirements in relation to the Building Code of Australia, dependent on the Bushfire Attack Level and requirements of Australian Standard 3959.
- Reduction and removal of vegetation and fuel loads in and around the property, buildings and Hazard Management Areas,
- Ongoing maintenance responsibilities by successive owners for perpetuity.

A copy of the plan MUST also be provided to ALL current and successive owners to make them aware of their continuing obligations to maintain the plan and protection measures attributed to their property in to the future.

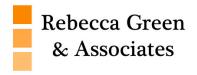
2.0 Hazard Management Areas

The Hazard Management Area (defendable space) is provided between the vegetation and the buildings subject to bushfire risk. The space provides for management of vegetation and reduction in fuel loads in an attempt to:

- · Prevent flame impingement on the dwelling;
- Provide a defendable space for property protection;
- Reduce fire spread;
- Deflect and filter embers;
- Provide shelter from radiant heat; and
- Reduce wind speed.

The *Building Act 2016*, requires a hazard management area to be established <u>and maintained</u> between the bushfire prone vegetation and the building at a distance equal to, or greater than the separation distance specified for the Bushfire Attack Levels (BAL) in *AS 3959-2018 Construction of Buildings in Bushfire Prone Areas*.

Refer to the attached BHMP Site Plan in Section 6 of this management plan for specific details on the Hazard Management Area.



2.1 Vegetation (Fuel) Management

Managing an area in a minimum fuel condition generally means a reduction in the amount and altering the arrangement of fuels. Most fine fuels are at or close to the ground, often as part of a grass, litter or shrub layer. If there is enough fuel, when a fire comes these fuels will ignite the trees above or set the bark alight which will burn up into the tree canopy causing the most dangerous of bushfire situation; a crown fire.

To prevent crown fires occurring it is necessary to remove the "ladder of fuel" between the ground and the tree crowns and to make sure the amount of ground fuel is not sufficient to set the crowns alight. Without fire burning below, a crown fire should not be sustained. Further removing continuity and separation of the vegetation canopies both horizontally and vertically will assist.

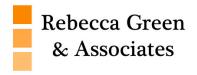
All vegetation will burn under the influence of bushfire; shrub layers need to be modified to remove tall continuous walls of vegetation and establish clear separation between the ground and the bottom of the tree canopy. Further minimisation of flammable ground litter such as leafs, twigs, bark, ferns and debris will further reduce fuel load with potential to burn or contribute to the growth of a bushfire.

Fuels do not need to be totally removed however fuels close to the building and inside the Hazard Management Area are to be kept to a minimum. As a general practice 5 tonnes per hectare is accepted as being controllable with normal firefighting resources. This can be visualised as grass cut to about 10 centimetres in height or ground litter about 2 centimetres thick. This is considered to be a low fuel level.

2.2 Other Risk Management Actions

Other actions that can be implemented to reduce the bushfire risk in the Hazard Management Areas include:

- 1. Establishing non-combustible paths and driveways around buildings.
- 2. Establish plantings of low flammability shrub species.
- 3. Ensure garden beds and shrubs are established well away from buildings.
- 4. Tree planting to be located at the outer edge of the Hazard Management Area and spaced well apart to ensure canopy separation.
- 5. Cut lawns short and maintain.
- 6. Remove fallen limbs, leaf and bark litter.
- 7. Avoid using pine bark and other flammable mulch in gardens.
- 8. Prune trees to ensure canopy separation horizontally and vertically, remove low hanging branches to ensure separation from ground litter.
- 9. Where the amount of land permits extend the vegetation management in to a secondary hazard management zone.



3.0 On-going Site Management and Maintenance

On-going maintenance is required to the buildings and landscaping within the hazard management area to ensure the continued performance of the bushfire mitigation measures which have been designed into the development for occupant and community protection.

Specified Hazard Management Areas are only a minimum distance required; owners are encouraged to establish a greater management area where land area and opportunity permits. An additional fuel modified buffer zone between the Hazard Management Area and the bushfire vegetation will only improve the protection level and reduce the risk to the property during a bushfire event.

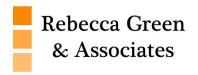
Preparedness comes down to diligent annual maintenance in and around the buildings and Hazard Management Areas particularly during the period of greatest risk; August to February of each year.

Recommendation:

- 1. Locate wood piles or other flammable storage well away from the dwelling.
- 2. Solid non-combustible fencing such as steel provides a fire and heat radiation shield to the dwelling.
- 3. Metal flywire screens prevent sparks and embers from entering the building.
- 4. Seal gaps under floor spaces, roof space, under eaves, external vents, skylights, chimneys and wall cladding.
- 5. Remove ladder fuels from the under storey of larger trees. Prune canopies to provide separation.
- 6. Rake up leaf litter and vegetation debris. Cut grass and maintain to less than 10cm.
- 7. Keep garden beds well away from the dwelling and use non-combustible garden mulches including rock or stones.
- 8. Establish plantings of low flammability shrub species.
- 9. Seal all gaps in external claddings.
- 10. Keep roof gutters clear of leaf litter, bark and similar debris, remove and maintain. Install gutter guards to assist.
- 11. Flammable fuels such as gas bottles should be located on the opposite side of the house to the likely direction of a bushfire.
- 12. Seal gaps in roofing to prevent the entry of embers.
- 13. Surround the dwelling with non-combustible paths.
- 14. Outbuildings to be at least 6m from the main dwelling.
- 15. Ensure hoses provide coverage to the whole site. Use metal hose fittings.
- 16. Flammable fuels and the like to be stored in minimum volumes well away from the dwelling.

4.0 Vehicular Access

Roads are to be constructed to provide vehicle access to the site to assist firefighting and emergency personnel to defend the building or evacuate occupants; and provide access at all times to the water supply for firefighting purposes on the building site.



Private access roads are to be constructed from the entrance to the property cross over with the public road through to the dwelling and water storage area on the site (if applicable). Private access roads are to be designed, constructed and maintained to a standard as recommended below:

Recommendations:

The following design and construction requirements apply to property access length is 30 metres or greater or access for a fire appliance to a fire fighting water point:

- (i) All weather construction;
- (ii) Load capacity of at least 20 tonnes, including for bridges and culverts;
- (iii) Minimum carriageway width of 4 metres;
- (iv) Minimum vertical clearance of 4 metres;
- (v) Minimum horizontal clearance of 0.5 metres from the edge of the carriageway;
- (vi) Cross falls of less than 3 degrees (1:20 or 5%);
- (vii) Dips less than 7 degrees (1:8 or 12.5%) entry and exit angle;
- (viii) Curves with a minimum inner radius of 10 metres;
- (ix) Maximum gradient of 15 degrees (1:3.5 or 28%) for sealed roads, and 10 degrees (1:5.5 or 18%) for unsealed roads; and
- (x) Terminate with a turning area for fire appliances provided by one of the following:
 - a) A turning circle with a minimum inner radius of 10 metres;
 - b) A property access encircling the building; or
 - c) A hammerhead "T" or "Y" turning head 4 metres wide and 8 metres long.

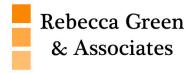
5.0 Water Supply

A building that is constructed in a designated bushfire prone area must provide access at all times to a sufficient supply of water for firefighting purposes on the building site.

Recommendations:

The exterior elements of a habitable building in a designated Bushfire prone area must be within reach of a 120m long hose (reticulated) or 90m long hose (static) (lay) connected to –

- (i) A fire hydrant system designed and constructed in accordance with TasWater Supplement to Water Supply Code of Australia WSA 03-2011-3.1 MRWA Edition 2.0; or
- (ii) A stored water supply in a water tank, swimming pool, dam or lake available for fire fighting at all times which has the capacity of at least 10,000L for each separate building area to be protected.



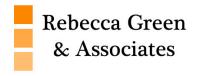
5.1 Reticulated Water Supply

Not applicable to this proposal.

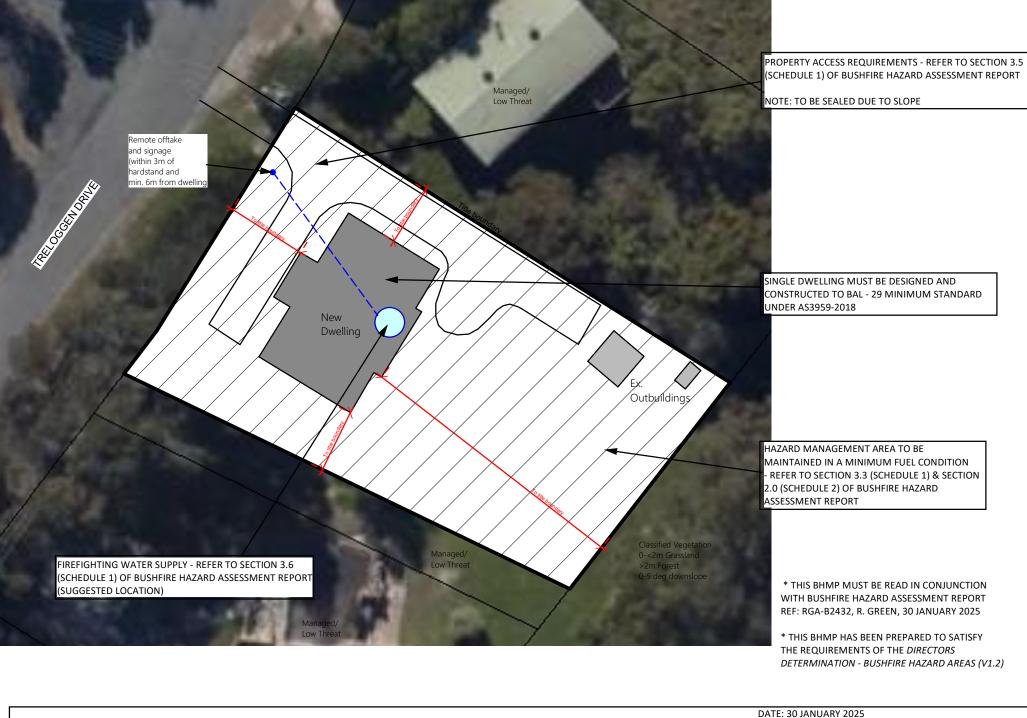
5.2 On-Site Dedicated Fire Fighting Water Supply

A water tank of at least 10,000 litres per building area to be protected and above ground pipes and fittings used for a stored water supply must be made of non-rusting, non-combustible, non-heat-deforming materials and must be situated more than 6m from a building, but within 90m of the building area (water connection point). Hardstanding must be provided within 3m of a static water supply/water connection point.

The water tank must be fitted with a 65mm outlet and DIN or NEN Standard compliant forged Storz 65mm adaptor fitted with a standard (delivery) washer rated to 1800kPa working pressure and 2400kPa burst pressure.



Bushfire Hazard Management Site Plan



BUSHFIRE HAZARD MANAGEMENT PLAN BUSHFIRE ATTACK LEVEL (BAL) - 29 **NEW SINGLE DWELLING**

60 TRELOGGEN DRIVE, BINALONG BAY **VOLUME 104144 FOLIO 1 PROPERTY ID 6798471**

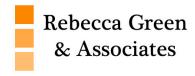
VERSION: 1

DRAWN: REBECCA GREEN

PHONE: 0409 284 422 EMAIL: ADMIN@RGASSOCIATES.COM.AU BFP - 116, SCOPE - 1, 2, 3A, 3B, 3C

Rebecca Green & Associates

N



Form 55

CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM Section 321 Owner /Agent To: Katie Ford Address 7 Middle Head Road Suburb/postcode MOSMAN NSW 2088 Qualified person details: Qualified person: Rebecca Green Phone No: Address: PO Box 2108 0409 284 422 Launceston 7250 Fax No: Licence No: **BFP-116** Email address: admin@rgassociates.com.au (description from Column 3 of the Director's Accredited to report on bushfire Determination - Certificates by Qualified Persons for hazards under Part IVA of the Fire Assessable Items Qualifications and Services Act 1979 Insurance details: (description from Column 4 of the Director's Analysis of hazards in bushfire prone Speciality area of Determination - Certificates by Qualified Persons for expertise: areas Assessable Items) **Details of work:** 60 Treloggen Drive Address: 1 Lot No: **BINALONG BAY TAS** 7216 Certificate of title No: 104144 (description of the assessable item being The assessable item **New Single Dwelling** certified) related to this Assessable item includes – certificate: a material: a design a form of construction a document testing of a component, building system or plumbing system an inspection, or assessment, performed **Certificate details:** (description from Column 1 of Schedule 1 of the **Bushfire Hazard** Certificate type: Director's Determination - Certificates by Qualified Persons for Assessable Items n) This certificate is in relation to the above assessable item, at any stage, as part of - (tick one) building work, plumbing work or plumbing installation or demolition work: a building, temporary structure or plumbing installation:

In issuing this certificate the following matters are relevant -

Bushfire Hazard Assessment Report &
Bushfire Hazard Management Plan (Rebecca Green & Associates, 30 January
2025, Version 1, Job No. RGA-B2432)

Relevant N/A

References: Australian Standard 3959-2018

Substance of Certificate: (what it is that is being certified)

- 1. Assessment of the site Bushfire Attack Level (BAL 29 for Single Dwelling) to Australian Standard 3959-2018
- 2. Bushfire Hazard Management Plan showing BAL-29 solutions.

Scope and/or Limitations

Scope

This report and certification was commissioned to identify the Bushfire Attack Level for the existing property. <u>All</u> comment, advice and fire suppression measures are in relation to compliance with the *Building Act 2016 & Regulations 2016*, *National Construction Code* and *Australian Standard 3959-2018*, *Construction of buildings in bushfire-prone areas*.

Limitations

The assessment has been undertaken and report provided on the understanding that:-

- 1. The report only deals with the potential bushfire risk all other statutory assessments are outside the scope of this certificate.
- 2. The report only identifies the size, volume and status of vegetation at the time the inspection was undertaken and cannot be relied upon for any future development.
- 3. Impacts of future development and vegetation growth have not been considered.
- 4. No assurance is given or inferred for the health, safety or amenity of the general public, individuals or occupants in the event of a Bushfire.
- 5. No warranty is offered or inferred for any buildings constructed on the property in the event of a Bushfire.

No action or reliance is to be placed on this certificate or report; other than for which it was commissioned.

I certify the matters described in this certificate.

| | Signed: | Certificate No: | Date: |
|-------------------|---------|-----------------|--------------------|
| Qualified person: | MGreen | RG-011/2025 | 30 January 2025 |



Attachment 1 – AS3959-2018 Construction Requirements

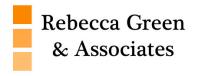
Rebecca Green & Associates

BAL Assessments

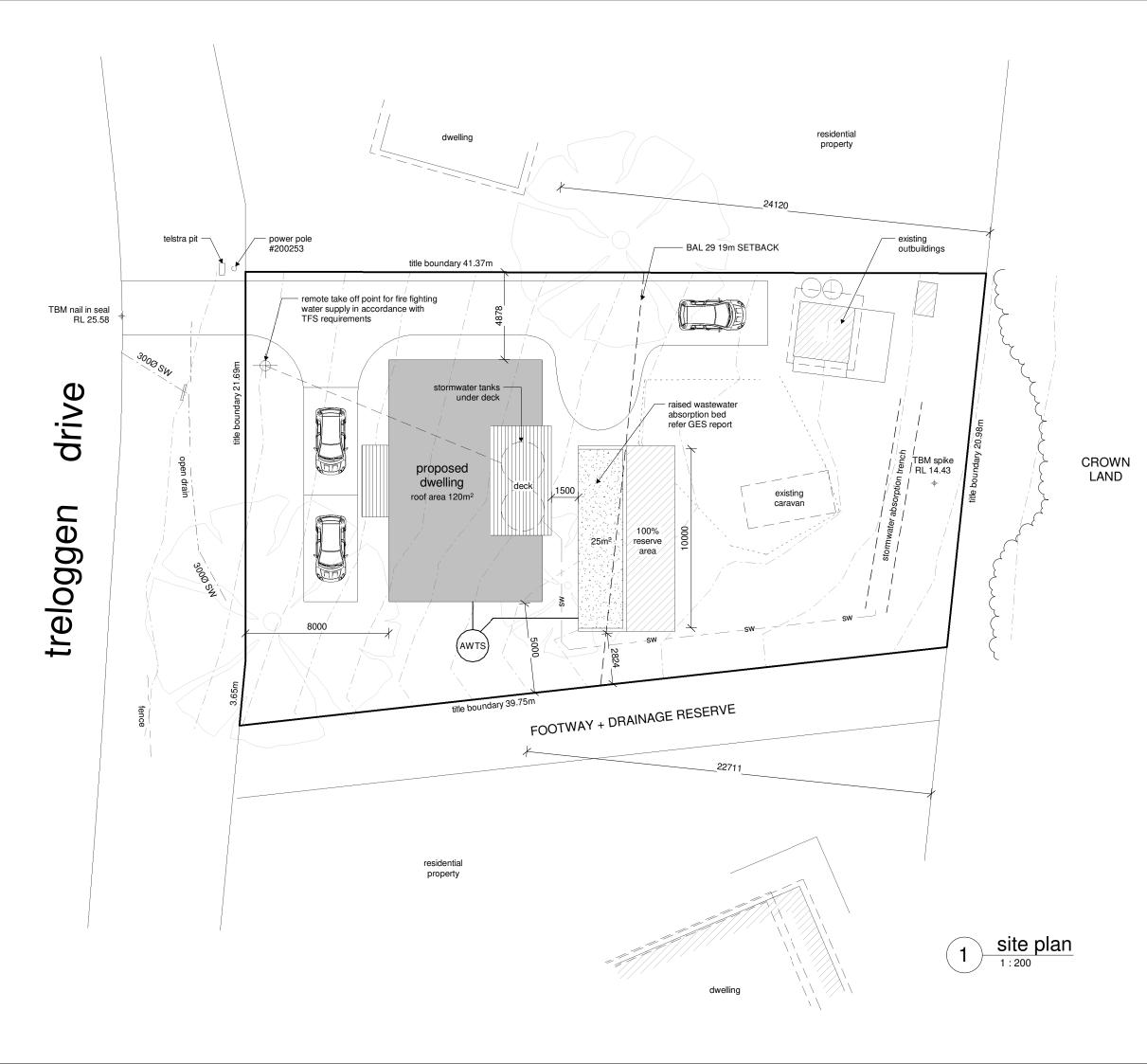
Revised for 2018 edition

| | BAL-LOW | BAL-12.5 | BAL-19 | BAL-29 | BAL-40 | BAL –FZ (FLAMEZONE) |
|----------------------|--------------------------------------|---|--|--|--|---|
| SUBFLOOR SUPPORTS | No special construction requirements | No special construction requirements | Enclosure by external wall or by steel, bronze or aluminium mesh | Enclosure by external wall or by steel, bronze of aluminium mesh. Non-combustible or naturally fire resistant timber supports where the subfloor is unenclosed | If enclosed by external wall refer below "External Walls" section in table or non-combustible sub-floor supports, or tested for bushfire resistance to AS1530.8.1 | Enclosure by external wall or non-combustible with an FRL of 30/-/- or to be tested for bushfire resistance to AS1530.8.2 |
| FLOORS | No special construction requirements | No special construction requirements | Concrete slab on ground or enclosure by external wall, metal mesh as above or flooring less than 400mm above ground level to be non-combustible, naturally fire resistant timber or protected on the underside with sarking or mineral wool insulation | Concrete slab on ground or enclosure by external wall, metal mesh as above or flooring less than 400mm above ground level to be non-combustible, naturally fire resistant timber or protected on the underside with sarking or mineral wool insulation | Concrete slab on ground or enclosure by external wall or protection of underside with a non-combustible material such as fibre cement sheet or be non-combustible or to be tested for bushfire resistance to AS1530.8.1 | Concrete slab on ground or enclosure by external wall or an FRL of 30/30/30 or protection of underside 30 minute incipient spread of fire system or to be tested for bushfire resistance to AS1530.8.2 |
| EXTERNAL WALLS | No special construction requirements | As for BAL-19 | Parts less than 400mm above ground or decks etc to be of non-combustible material, 6mm fibre cement clad or bushfire resistant/ naturally fire resistant timber | Non-combustible material (masonry, brick veneer, mud brick, aerated concrete, concrete) or timber framed, or steel framed walls sarked on the outside and clad with 6mm fibre cement sheeting or steel sheeting or bushfire resistant timber | Non-combustible material (masonry, brick veneer, mud brick, aerated concrete, concrete) or timber framed, or steel framed walls sarked on the outside and clad with 9mm fibre cement sheeting or steel or to be tested for b | Non-combustible material (masonry, brick veneer, mud brick, aerated concrete, concrete) with a minimum thickness of 90mm or a FRL of -/30/30 when tested from outside or to be tested for bushfire resistance to AS1530.8.2 |
| EXTERNAL WINDOWS | No special construction requirements | 4mm grade A Safety Glass of glass blocks within 400m of ground, deck etc with Openable portion metal screened with frame of metal or metal reinforced PVC-U or bushfire resisting timber | 5mm toughened glass or glass bricks within 400mm of the ground, deck etc with openable portion metal screened with frame of metal or metal reinforced PVC-U or bushfire resisting timber. Above 400mm annealed glass can be used with all glass screened | 5mm toughened glass with openable portion screened and frame of metal or metal reinforced PVC-U, or bushfire resistant timber and portion within 400mm of ground, deck, screen etc screened | 6mm toughened glass. Fixed and openable portion screened with steel or bronze mesh | Protected by bushfire shutter or FRL of -/30/- and openable portion screened with steel or bronze mesh or be tested for bushfire resistance to AS1530.8.2 |
| EXTERNAL DOORS | No special construction requirements | As for BAL-19 except that door framing can be naturally fire resistant (high density) timber | Screened with steel, bronze or aluminium mesh or glazed with 5mm toughened glass, non-combustible or 35mm solid timber for 400mm above threshold, metal or bushfire resistant timber framed for 400mm above ground, decking etc. tight-fitting with weather strips at base | Screened with steel, bronze or aluminium mesh or non-combustible, or 35mm solid timber for 400mm above threshold. Metal or bushfire resistant timber framed tight-fitting with weather strips at base | Non-combustible or 35mm solid timber, screened with steel or bronze mesh, metal framed, tight-fitting with weather strips at base | Protected by bushfire shutter or tight-fitting with weather strips at base and a FRL of -/30/- |
| ROOFS | No special construction requirements | As for BAL-19 (including roof to be fully sarked) | Non-combustible covering, roof/wall junctions sealed. Openings fitted with non-combustible ember guards. Roof to be fully sarked. | Non-combustible covering. Roof/wall junction sealed. Openings fitted with non-combustible ember guards. Roof to be fully sarked | Non-combustible covering. Roof/wall junction sealed. Openings fitted with non-combustible ember guards. Roof to be fully sarked and no roof mounted evaporative coolers | Roof with FRL of 30/30/30 or tested for bushfire resistance to AS1530.8.2. Roof/wall junction sealed. Openings fitted with non-combustible ember guards. No roof mounted evaporative coolers |
| VERANDAS DECKS ETC. | No special construction requirements | As for BAL-19 | Enclosed sub floor space—no special requirements for materials except within 400mm of ground. No special requirements for supports or framing. Decking to be non-combustible or bushfire resistant within 300mm horizontally and 400mm vertically from a glazed element | Enclosed sub floor space or non-combustible or bushfire resistant timber supports. Decking to be non-combustible or bushfire resistant timbers | Enclosed sub-floor space or non-combustible supports. Decking to be non-combustible | Enclosed sub floor space or non-combustible supports. Decking to have no gaps and be non-combustible |

Please note: The information in the table is a summary of the construction requirements in the AS3959-2018 standard and is not intended as a design or construction guide. You should consult the standard for the full technical details.



Attachment 2 – Proposal Plans





| 1 | Revision 1 | Date 1 |
|-------|--------------|--------|
| REV: | DESCRIPTION: | DATE: |
| PROJE | ECT: | |

proposed dwelling

FOR:

k. ford60 treloggen drivebinalong bay TAS 7216

DRAWING TITLE:

site plan

| DRAWING NO: | DRAWN BY: JB | |
|--------------|-----------------|--|
| a04 | DATE: 23.01.25 | |
| SCALE: 1:200 | PROJECT: 1123FO | |



www.jenniferbinnsdesign.com.au 0439 765 452 : mail @ jenniferbinnsdesign.com.au 52 cecilia street st helens tasmania 7216



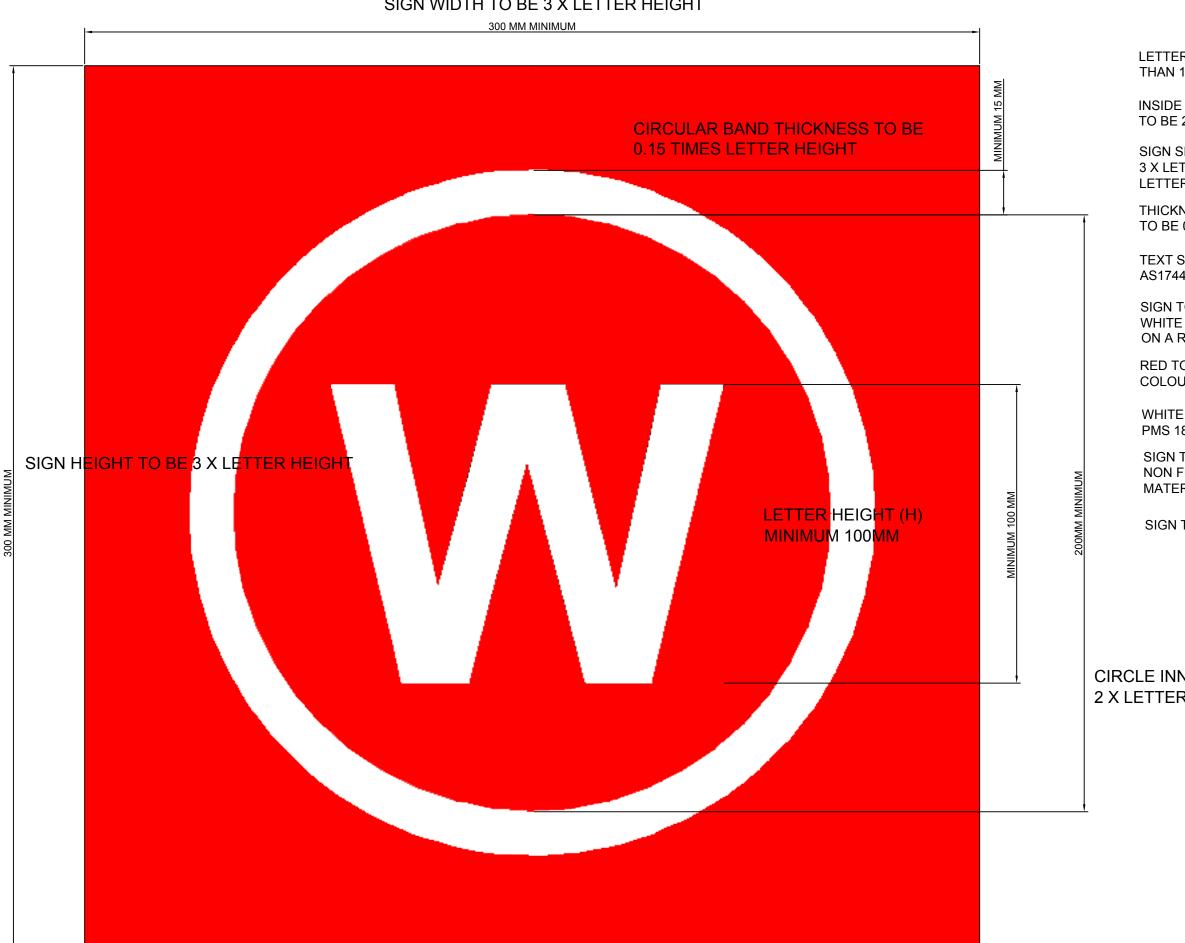
ACCREDITATION NO: CC 1269L



Attachment 3 – Tasmania Fire Service Water Supply Signage Guideline

10,000 LITRE DOMESTIC FIREFIGHTING STATIC WATER INDICATOR SIGN

SIGN WIDTH TO BE 3 X LETTER HEIGHT



LETTERING TO BE UPPERCASE AND NOT LESS THAN 100MM IN HEIGHT

INSIDE DIAMETER OF CIRCULAR BAND TO BE 2 TIMES LETTER HEIGHT

SIGN SIZE DIMENSIONS 3 X LETTER HEIGHT HIGH AND 3 X LETTER HEIGHT WIDE.

THICKNESS OF CIRCULAR BAND TO BE 0.15 TIMES LETTER HEIGHT

TEXT STYLE TO BE IN ACCORDANCE WTH AS1744.2015, SERIES F

SIGN TO BE IN FADE RESISTING MATERIAL WITH WHITE REFLECTIVE LETTERING AND CIRCLE ON A RED BACKGROUND

RED TO BE R-13 SIGNAL RED COLOUR CODE 1795U

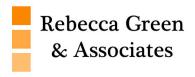
WHITE SUBSTRATE COLOUR TO BE PMS 186C

SIGN TO BE CONSTRUCTED FROM UV STABILIZED, NON FLAMMABLE AND NON HEAT DEFORMING MATERIAL

SIGN TO BE PERMANENTLY FIXED

CIRCLE INNER DIAMETER 2 X LETTER HEIGHT





References

- (a) Australian Standards, AS 3959-2018, *Construction of buildings in bushfire-prone areas*, Standards Australia, Sydney NSW.
- (b) Resource Management & Conservation Division of the Department Primary Industry & Water September 2006, TASVEG, *Tasmanian Vegetation Map*, Tasmania.
- (c) Tasmanian Government, Land Information System Tasmania, www.thelist.tas.gov.au

STORMWATER ASSESSMENT

60 Treloggen Drive Binalong Bay January 2025



Disclaimer: The author does not warrant the information contained in this document is free from errors or omissions. The author shall not in any way be liable for any loss, damage or injury suffered by the User consequent upon, or incidental to, the existence of errors in the information.



Investigation Details

Client: Katie Ford

Site Address: 60 Treloggen Drive, Binalong Bay

Date of Inspection: 28/01/2025

Proposed Works: New house

Investigation Method: Hand Auger

Inspected by: JP Cumming

Site Details

Certificate of Title (CT): 104144/1

Title Area: Approx. 928.6 m²

Applicable Planning Overlays: Bushfire-prone areas, Landslip Hazard, Priority Vegetation

Slope & Aspect: 7° SE facing slope

Vegetation: Mixed Flora

Background Information

Geology Map: MRT

Geological Unit: Devonian Granite

Climate: Annual rainfall 700mm

Water Connection: Tank

Sewer Connection: Unserviced-On-site required

Testing and Classification: Onsite stormwater retention



Investigation

A number of bore holes were completed to identify the distribution and variation of the soil materials at the site, bore hole locations are indicated on the site plan. See soil profile conditions presented below.

Soil Profile Summary

| BH 1 Depth (m) | BH 2 Depth (m) | uscs | Description |
|-------------------|-------------------|------|---|
| 0.0 – 0.40 | 0.0 – 0.30 | SW | Dark Grey SAND , slightly moist, medium dense consistency, trace of gravels, common fine rootlets, gradual boundary to |
| 0.40 - 0.90 | 0.30 – 0.80 | SW | Light Brownish Grey SAND , slightly moist to moist, medium dense consistency, trace of gravels, clear boundary to |
| 0.90 – 1.10 | 0.80 – 1.00 | SW | Black SAND , dry, dense consistency, ~10% quartz gravels, clear boundary to |
| 1.10 – 1.8+ | 1.00 – 1.5+ | SP | Light Brown SAND , dry, dense consistency, ~10% quartz gravels |

Soil Conditions

The soil on site has developed from windblown sands overlying Devonian granite. The soil has an estimated permeability of approximately >3m/day

GES have identified the following at the site:

- There are no proposals for cuts or changes of grade which may impact on any proposed onsite stormwater absorption.
- The soil onsite has been identified as comprising of sands. No soil dispersion was identified.
- No evidence of a water table was observed at the time of the investigation
- There is a low risk of the natural soils being impacted by contamination
- Bedrock was not encountered within 1.8m

Soil Dispersion

The soil is non-dispersive.



Existing Conditions and Assumptions

The site covers an area of approximately 928m² with a total proposed roof area of approx. 120m² and proposed office with area of approx. 180m².

There is no public stormwater system that the property can connect to, and it is therefore it is proposed that stormwater from the site would be routed through the proposed conventional underground drainage system comprising of Grated Sumps and PVC Pipes, coupled with soakage trench elements for on-site detention. The stormwater management report is prepared in accordance with the design criteria listed below:

- The stormwater drainage system is designed using Bureau of Meteorology (BOM) published rainfall Intensity Frequency Duration (IFD) data as a minor / major system to accommodate the 5% AEP / 20 min storm events.
- The flow rate of stormwater leaving the site shall be designed so that it does not exceed the pre- developed flow rate for both the minor and major rain events.
- The total site discharges are modelled as described in Storm Drainage Design in Small Urban Catchments, a handbook for Australian practice by Australian Rainfall and Runoff (ARR2019), Book 9 – Runoff in Urban Areas.

Detention Calculations

Detention calculations area provided in Appendix A

Summary and Conclusions

- Detention design to be adopted as per design and documentation.
- The designed solution complies with the performance solution design check carried out.
- The 18m² base (12m x 1.5m), 1.2m deep soakage trench is designed over a 20-minute storm duration for proposed development.
- DN100 slotted PVC pipe with geotextile covering on top of aggregate to be installed within the soakage trench.

It is also recommended that regular inspection and maintenance is conducted to ensure the stormwater system is operating without obstruction. A schematic of recommended checks is attached.



GES Stormwater Maintenance Plan Checklist

| Indicative frequency | Inspection and criteria | Maintenance activities (where required) |
|----------------------|---|---|
| Annual | Check whether any tree branches overhang the roof or are likely to grow to overhang the roof | If safe and where permitted, consider pruning back any overhanging branches |
| | Check that access covers to storage tanks are closed | Secure any open access covers to prevent risk of entry |
| | Check that screens on inlets, overflows and other openings do not have holes and are securely fastened | Repair any defective screens to keep out mosquitoes |
| | Inspect tank water for presence of rats, birds, frogs, lizards or other vermin or insects | Remove any infestations, identify point of entry and close vermin and insect-proof mesh |
| | Inspect tank water for presence of mosquito larvae (inspect more frequently in sub-tropical and tropical northern Australia, based on local requirements) | Identify point of entry and close with insect-proof mesh with holes no greater than 1.6 mm in diameter |
| | Inspect gutters for leaf accumulation and ponding | Clean leaves from gutters-remove more regularly if required. If water is ponding, repair gutter to ensure water flows to downpipe |
| | Check signage at external roof water taps and that any removable handle taps are being properly used | Replace or repair the missing or damaged signage and fittings |
| | Check plumbing and pump connections are watertight/without leakage | Repair any leaks as necessary |
| | Check suction strainers, in-line strainers and pump location for debris | Clean suction strainers, in-line strainers or debris from pump location |
| | Check pump installation is adequate for reliable ongoing operation | Modify and repair as required |
| | Check first flush diverter, if present | Clean first flush diverter, repair and replace if necessary |
| | Check health of absorption trench area and surrounding grass or plants | Investigate any adverse impacts observed that might be due to irrigation |
| | Check condition of roof and coatings | Investigate and resolve any apparent changes to roof condition, such as loss of material coatings |







| Triennial | Drain, clean out and check the condition of the tank walls and roof to ensure no holes have arisen due to tank deterioration | Repair any tank defects |
|---|--|---|
| | Check sediment levels in the tank | Organise a suitable contractor to remove accumulated sediment if levels are approaching those that may block tank outlets |
| | Undertake a systematic review of operational control of risks to the system | Identify the reason for any problems during inspections and take actions to prevent failures occurring in future |
| After 20 years and then every 5 years | Monitor the effectiveness of the stormwater absorption area to assess for any clogging due to algal growth, or blocking due to tree roots/grass growth/trench failure. | Clean or replace clogged equipment |
| Ongoing | Inspect and follow up on any complaints or concerns raised that could indicate problems with the system | Repair or replace any problems that are notified |







APPENDIX A: STORMWATER DETENTION CALCULATIONS

| STORAGE TRENCH | | | | | | |
|--------------------------|--------------------|--------|--------|--|--|--|
| | | | | | | |
| Hydrology | | | | | | |
| Total Catchment A | rea | 300 | m2 | | | |
| Runoff Coefficient | | 0.94 | | | | |
| Annunal Recurrence | e Interval (ARI) | 20 | yr | | | |
| Ground Conditions | | | | | | |
| Hydraulic conducti | vity (K) | 5 | m/day | | | |
| | | 3.470 | mm/min | | | |
| Adjusted Rate (15% | 6 clogging factor) | 2.950 | mm/min | | | |
| Trench Design | | | | | | |
| Length | | 12 | m | | | |
| Width | | 1.5 | m | | | |
| Depth | | 1.2 | m | | | |
| Infiltration Area | | 18 | m2 | | | |
| Porosity | | 0.35 | % | | | |
| Trench Storage | | 7.56 | m3 | | | |
| | | 7560 | L | | | |
| | | | | | | |
| Final Check | | | | | | |
| Criteria | Requirement | Design | Check | | | |
| Detention reqd | 6770 | 7560 | OK | | | |



| FORM CHECK | | | | | |
|---------------------------|----------------|-------------------------|-----------------|---------------------|--------------|
| Storm Duration | Intensity | Inflow Volume | Outflow Volume | Required Storage | Emptying tim |
| | (mm/hr) | (m³) | (L) | (L) | (hr) |
| 1 min | 216 | 1015 | 53 | 962 | 0.30 |
| 2 min | 167 | 1570 | 106 | 1464 | 0.46 |
| 3 min | 153 | 2157 | 159 | 1998 | 0.63 |
| 4 min | 143 | 2688 | 212 | 2476 | 0.78 |
| 5 min | 134 | 3149 | 265 | 2884 | 0.91 |
| 10 min | 103 | 4841 | 531 | 4310 | 1.35 |
| 15 min | 83.9 | 5915 | 796 | 5119 | 1.61 |
| 20 min | 71.4 | 6712 | 1062 | 5650 | 1.77 |
| 25 min | 62.5 | 7344 | 1327 | 6016 | 1.89 |
| 30 min | 55.9 | 7882 | 1593 | 6289 | 1.97 |
| 45 min | 43.4 | 9179 | 2389 | 6790 | 2.13 |
| 1 hour | 36.4 | 10265 | 3185 | 7079 | 2.22 |
| 1.5 hour | 28.6 | 12098 | 4778 | 7320 | 2.30 |
| 2 hour | 24.4 | 13762 | 6371 | 7391 | 2.32 |
| 3 hour | 19.7 | 16666 | 9556 | 7110 | 2.23 |
| 4.5 hour | 16.2 | 20558 | 14335 | 6223 | 1.95 |
| 6 hour | 14.2 | 24026 | 19113 | 4914 | 1.54 |
| 9 hour | 11.8 | 29948 | 28669 | 1279 | 0.40 |
| 12 hour | 10.3 | 34855 | 38226 | - | - |
| 18 hour | 8.32 | 42232 | 57338 | _ | - |
| 24 hour | 7.05 | 47714 | 76451 | _ | - |
| 30 hour | 6.12 | 51775 | 95564 | _ | - |
| 36 hour | 5.4 | 54821 | 114677 | - | - |
| 48 hour | 4.36 | 59017 | 152902 | _ | - |
| 72 hour | 3.11 | 63145 | 229353 | - | - |
| | | | Full volume | 7560 | 2.32 |
| lotes: | | | | | |
| nflow volume calculated | using Fauation | IO 1 (WSUD Guidelin | es: Chanter 10) | | |
| Outflow volume calculated | <u> </u> | • | | | |







Location

Label: 60 Treloggen Dr Binalong Bay

Easting: 610075 Northing: 5432030 Zone: 55

Latitude: Nearest grid cell: 41.2625 (S)
Longitude:Nearest grid cell: 148.3125 (E)

IFD Design Rainfall Intensity (mm/h)

Issued: 29 January 2025

Rainfall intensity for Durations, Exceedance per Year (EV), and Annual Exceedance Probabilities (AEP), FAQ for New ARR probability terminology

Table

Chart

Unit: mm/h ❤

| | | Annu | ial Exceed | ance Prob | ability (A | EP) | |
|---------------|-------|-------|------------|-----------|------------|------|------|
| Duration | 63.2% | 50%# | 20%* | 10% | 5% | 2% | 1% |
| 1 <u>min</u> | 97.9 | 110 | 152 | 183 | 216 | 263 | 30: |
| 2 min | 82.0 | 91.9 | 124 | 146 | 167 | 195 | 215 |
| 3 <u>min</u> | 73.7 | 82.7 | 112 | 132 | 153 | 180 | 200 |
| 4 min | 67.3 | 75.7 | 103 | 123 | 143 | 170 | 19: |
| 5 min | 62.2 | 70.0 | 95.9 | 115 | 134 | 161 | 183 |
| 10 <u>min</u> | 46.0 | 51.9 | 71.9 | 87.0 | 103 | 126 | 146 |
| 15 <u>min</u> | 37.4 | 42.1 | 58.4 | 70.8 | 83.9 | 103 | 120 |
| 20 <u>min</u> | 31.9 | 36.0 | 49.8 | 60.3 | 71.4 | 87.6 | 101 |
| 25 <u>min</u> | 28.2 | 31.7 | 43.8 | 52.9 | 62.5 | 76.3 | 88.0 |
| 30 min | 25.4 | 28.6 | 39.4 | 47.4 | 55.9 | 68.0 | 78.0 |
| 45 <u>min</u> | 20.2 | 22.7 | 31.1 | 37.1 | 43.4 | 52.2 | 59.3 |
| 1 hour | 17.2 | 19.3 | 26.3 | 31.3 | 36.4 | 43.3 | 48. |
| 1.5 hour | 13.8 | 15.5 | 21.0 | 24.8 | 28.6 | 33.6 | 37.5 |
| 2 hour | 11.9 | 13.4 | 18.0 | 21.2 | 24.4 | 28,4 | 31.5 |
| 3 hour | 9.64 | 10.9 | 14.7 | 17.3 | 19.7 | 22.9 | 25.3 |
| 4.5 hour | 7.85 | 8.90 | 12.1 | 14.2 | 16.2 | 18.8 | 20. |
| 6 hour | 6.78 | 7.70 | 10.5 | 12.4 | 14.2 | 16.5 | 18.3 |
| 9 hour | 5.46 | 6.24 | 8.63 | 10.2 | 11.8 | 13.8 | 15.4 |
| 12 hour | 4.64 | 5.32 | 7.43 | 8.86 | 10.3 | 12.2 | 13.6 |
| 18 hour | 3.61 | 4.16 | 5.90 | 7.11 | 8.32 | 9.99 | 11.3 |
| 24 hour | 2.97 | 3,43 | 4.91 | 5.97 | 7.05 | 8.54 | 9.76 |
| 30 hour | 2.53 | 2.92 | 4.21 | 5.15 | 6.12 | 7.46 | 8.56 |
| 36 hour | 2.20 | 2.54 | 3.68 | 4.52 | 5,40 | 6.61 | 7.6 |
| 48 hour | 1.74 | 2.01 | 2.93 | 3.62 | 4.36 | 5.35 | 6.17 |
| 72 hour | 1.23 | 1.42 | 2.07 | 2.57 | 3.11 | 3.81 | 4.39 |
| 96 hour | 0.962 | 1.11 | 1.60 | 1.98 | 2.39 | 2.92 | 3.3 |
| 120 hour | 0.796 | 0.913 | 1.32 | 1.62 | 1.95 | 2.36 | 2.69 |
| 144 hour | 0.688 | 0.788 | 1.13 | 1.38 | 1.64 | 1.98 | 2.25 |
| 168 hour | 0.613 | 0.701 | 0.995 | 1.21 | 1.43 | 1.71 | 1.93 |

Note:

[#] The 50% AEP IFD does not correspond to the 2 year Average Recurrence Interval (ARI) IFD. Rather it corresponds to the 1.44 ARI.

^{*} The 20% AEP IFD does not correspond to the 5 year Average Recurrence Interval (ARI) IFD. Rather it corresponds to the 4.48 ARI.



60 Treloggen Drive Binalong Bay

Designed: 29/01/2025

Geo-Environmental Solutions

STORMWATER DETENTION V5.05

Location: Binalong Bay, Tas

Site: 300m² with tc = 20 and tcs = 15 mins.

PSD: AEP of 5%, Underground rectangular tank PSD = 1.79L/s Storage: AEP of 5%, Underground rectangular tank volume = 6.77m³

Design Criteria

(Custom AEP IFD data used)

Location = Binalong Bay, Tas

E (A)RI 2001,A(E)P 2019 Method =

PSD annual exceedance probabiliy (APE) = 5 % Storage annual exceedance probabiliy (APE) = 5 %

> Storage method = U (A)bove,(P)ipe,(U)nderground,(C)ustom

Site Geometry

300 m² = Site area (As) = 0.03 Ha Pre-development coefficient (Cp) = 0.30

Post development coefficient (Cw) = 0.94

Total catchment (tc) = 20 minutes Upstream catchment to site (tcs) = 15 minutes

Coefficient Calculations

Pre-development

| Zone | Area (m²) | С | Area * C |
|----------|-----------|------|----------|
| Concrete | 0 | 0.90 | 0 |
| Roof | 0 | 1.00 | 0 |
| Gravel | 0 | 0.50 | 0 |
| Garden | 300 | 0.30 | 90 |
| Total | 300 | m² | 90 |
| | | | |

Cp = ΣArea*C/Total = 0.300

Post development

| Zone | Area (m²) | С | Area * C |
|----------|-----------|------|----------|
| Concrete | 180 | 0.90 | 162 |
| Roof | 120 | 1.00 | 120 |
| Gravel | 0 | 0.50 | 0 |
| Garden | 0 | 0.30 | 0 |
| Total | 300 | m² | 282 |

0.940 Cw = ΣArea*C/Total =

Permissible Site Discharge (PSD) (AEP of 5%)

PSD Intensity (I) = 71.4 mm/hr For catchment tc = 20 mins.

Pre-development (Qp = Cp*I*As/0.36) = 1.78 L/s

Peak post development (Qa = 2*Cw*I*As/0.36) = 11.18 L/s $=(0.157 \times I)$ Eq. 2.24

> $U\ (A) bove, (P) ipe, (U) nder ground, (C) us to m\\$ Storage method =

Permissible site discharge (Qu = PSD) = 1.788 L/s

Above ground - Eq 3.8

 $0 = PSD^2 - 2*Qa/tc*(0.667*tc*Qp/Qa + 0.75*tc+0.25*tcs)*PSD + 2*Qa*Qp$

Taking x as = PSD and solving

a = 1.0 -23.3 39.9

 $PSD = -b\pm v(b^2-4ac)/(2a)$ 1.856 L/s

Below ground pipe - Eq 3.3

 $Qp = PSD^*[1.6*tcs/\{tc^*(1-2*PSD/(3*Qa))\}-0.6*tcs^{2\cdot67}/\{tc^*(1-2*PSDp/(3*Qa))\}^{2\cdot67}]$ 1.78 PSD = 1.842 L/s

Below ground rectangular tank - Eq 3.4

t = tcs/(tc*(1-2*PSD/(3*Qa))) =0.839

 $Qp = PSD^*[0.005-0.455*t+5.228*t^2-1.045*t^3-7.199*t^4+4.519*t^5]$

1.78 PSD = 1.788 L/s



Designed: 29/01/2025

60 Treloggen Drive Binalong Bay

STORMWATER DETENTION V5.05

Geo-Environmental Solutions

Design Storage Capacity (AEP of 5%)

Above ground (Vs) = $[0.5*Qa*td-[(0.875*PSD*td)(1-0.917*PSD/Qa)+(0.427*td*PSD^2/Qa)]]*60/10^3 m^3$ Eq 4.23 Below ground pipe (Vs) = $[(0.5*Qa-0.637*PSD+0.089*PSD^2/Qa)*td]*60/10^3 m^3$ Eq 4.8 Below ground rect. tank (Vs) = $[(0.5*Qa-0.572*PSD+0.048*PSD^2/Qa)*td]*60/10^3 m^3$ Eq 4.13

| td | I | Qa | Above Vs | Pipe Vs | B/G Vs |
|--------|---------|-------|----------|---------|--------|
| (mins) | (mm/hr) | (L/s) | (m³) | (m³) | (m³) |
| 5 | 134.3 | 21.0 | | | 2.85 |
| 25 | 62.5 | 9.8 | | | 5.83 |
| 36 | 49.9 | 7.8 | | | 6.28 |
| 46 | 42.8 | 6.7 | | | 6.50 |
| 56 | 38.0 | 5.9 | | | 6.64 |
| 66 | 34.3 | 5.4 | | | 6.72 |
| 76 | 31.6 | 4.9 | | | 6.76 |
| 87 | 29.2 | 4.6 | | | 6.77 |
| 97 | 27.4 | 4.3 | | | 6.76 |
| 107 | 25.9 | 4.1 | | | 6.73 |

Table 1 - Storage as function of time for AEP of 5%

| | td | İ | Qa | Vs |
|----------|--------|---------|-------|------|
| Туре | (mins) | (mm/hr) | (L/s) | (m³) |
| Above | | | | |
| Pipe | | | | |
| B/ground | 85.8 | 29.4 | 4.6 | 6.77 |

Table 2 - Storage requirements for AEP of 5%

Frequency of operation of Above Ground storage

| Qop2 = | 0.75 Cl 2.4.5.1 | |
|---------------------------------|---|---------|
| Qp2 =Qop2*Qp1 (where Qp1=PSD) = | 1.39 L/s at which time above ground storage occur | 'S |
| $I = 360*Qp2/(2*Cw*As*10^3) =$ | 8.9 mm/h | Eq 4.24 |

Period of Storage

Time to Fill:

| Above ground (tf) = $td*(1-0.92*PSD/Qa)$ | Eq 4.27 |
|--|---------|
| Below ground pipe (tf) = $td*(1-2*PSD/(3*Qa))$ | Eq 3.2 |
| Below ground rect. tank (tf) = $td*(1-2*PSD/(3*Qa))$ | Eq 3.2 |
| Time to empty: | |

Time to empty:

| Above ground (te) = (Vs+0.33*PSD ² *td/Qa*60/10 ³)*(1.14/PSD)*(10 ³ /60) | Eq 4.28 |
|---|---------|
| Below ground pipe (te) = 1.464/PSD*(Vs+0.333*PSD ² *td/Qa*60/10 ³)*(10 ³ /60) | Eq 4.32 |
| Below ground rect. tank (te) = $2.653/PSD*(Vs+0.333*PSD^2*td/Qa*60/10^3)*(10^3/60)$ | Eq 4.36 |

Storage period (Ps = tf + te) Eq 4.26

| | td | Qa | Vs | tf | te | Ps |
|----------|--------|-------|-------|--------|--------|--------|
| Туре | (mins) | (L/s) | (L/s) | (mins) | (mins) | (mins) |
| Above | | | | | | |
| Pipe | | | | | | |
| B/ground | 85.8 | 4.6 | 6.8 | 63.6 | 196.9 | 260.5 |

Table 3 - Period of Storage requirements for AEP of 5%

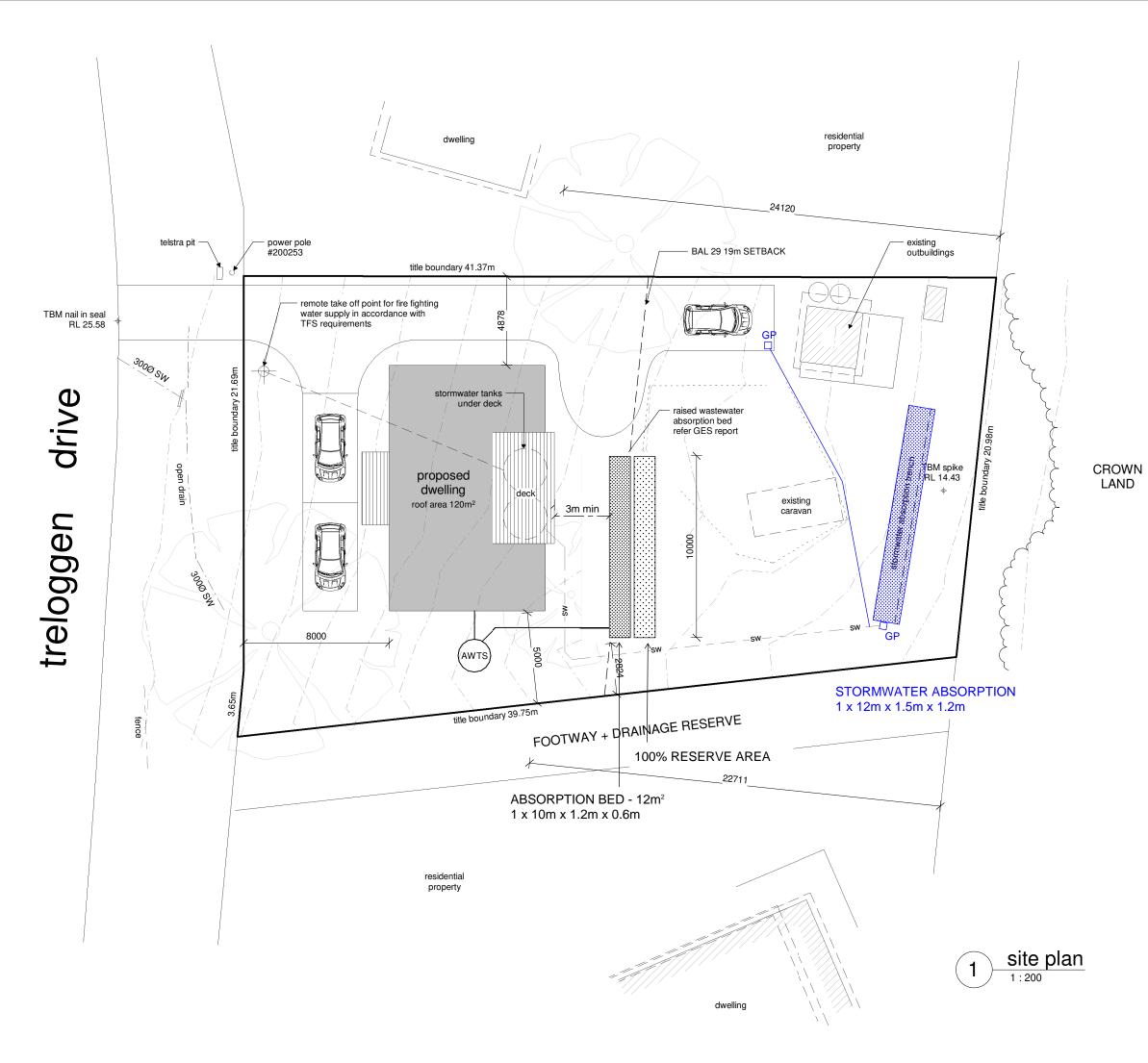
Orifice

Permissible site discharge (Qu=PSD) = 1.79 L/s (Underground storage) Orifice coefficient (CD) = 1 For sharp circular orifice Gravitational acceration (g) = 9.81 m/s²

Maximum storage depth above orifice (H) = 1200 mm Orifice flow (Q) = CD*Ao*V(2*g*H)

Therefore:

Orifice area (Ao) = 368 mm² Orifice diameter (D = $V(4*Ao/\pi)$) = 21.7 mm



New Services

STORMWATER PIPE WITH FLOW DIRECTION

GRATED STORMWATER PIT
450x450 CLASS A
ACO GALVANISED HEELGUARD OR SIMILAR
ENGINEER APPROVED

Performance Solution Compliance Notes:
A\$ 3500.3 · Ct. 7.10
· 7.10.1 · OVERFLOW IS SAFE AND DOES NOT COMPROMISE FREEBOADAD TO HABITABLE SPACES.
GENERAL
· ASINZS 3500.3 · PART 3 STORMWATER DRAINAGE AUSTRALIAN RAINFALL AND RUN-OFF VOLUME 8: URBAN STORMWATER MANAGEMENT
A MUSTRALIAN RUNOFF QUALITY - A GUIDE TO WATER SENSITIVE URBAN DESIGN
STORM DRAINAGE DESIGN IN SMALL URBAN CATCHMENTS:
A HANDBOOK FOR AUSTRALIAN PRACTICE
WATER SENSITIVE URBAN DESIGN (WSUD) ENGINEERING PROCEDURE: STORMWATER
• WATER SENSITIVE URBAN DESIGN (WSUD) WATER SENSITIVE URBAN DESIGN (WSUD) ENGINEERING PROCEDURE: STORMWATER

Stormwater Services Notes:

1. ALL SITE SAFETY & MANAGEMENT PROCEDURES SHALL BE IN ACCORDANCE WITH THE DEPARTMENT OF STATE GROWTH SPECIFICATIONS:
SECTION 168 OCCUPATIONAL HEALTH AND SAFETY & SECTION 176 ENVIRONMENTAL MANAGEMENT.

2. ALL PIPES UNDER TRAFFICABLE AREAS ARE TO BE BACKFILLED

FULL DEPTH WITH 20 F.C.R. AND FULLY COMPACTED.

3. ALL STORMWATER PIPES TO BE PVC-U-SWJ CLASS 'SN8" TO AS1254 UNO.

4. ALL DRAIN AND TRENCH CONSTRUCTION SHALL COMPLY WITH THE LGAT STANDARD DRG TSD G01.

5. ANY EXCAVATED TRENCHES IN EXCESS OF 1.5M IN DEPTH ARE TO BE ADEQUATELY SHORED TO PREVENT COLLAPSE DURING WORKS.



| 1 | Revision 1 | Date 1 |
|------|--------------|--------|
| REV: | DESCRIPTION: | DATE: |

PROJECT:

proposed dwelling

k. ford

60 treloggen drive binalong bay TAS 7216

DRAWING TITLE:

site plan

| DRAWING NO: | | DRAWN BY: JB | | | |
|-------------|--------------|-----------------|--|--|--|
| | a04 | DATE: 23.01.25 | | | |
| | SCALE: 1:200 | PROJECT: 1123FO | | | |



www.jenniferbinnsdesign.com.au 0439 765 452 : mail @ jenniferbinnsdesign.com.au 52 cecilia street st helens tasmania 7216



ACCREDITATION NO: CC 1269L

CERTIFICATE OF THE RESPONSIBLE DESIGNER

Section 94 Section 106 Section 129 Section 155

| To: | Katie Ford | Owner name | 25 | | | |
|---------------------|---|---------------------------------------|---------------------|--|--|--|
| | 10 Hilly Street | Address | Form 35 | | | |
| [| Mortlake | Suburb/postcode | | | | |
| Designer details | e: | | | | | |
| | 5. | | | | | |
| Name: | Vinamra Gupta | | Category: | Civil Engineer | | |
| Business name: | Geo-Environmental Solutions | 5 | Phone No: | 03 6223 1839 | | |
| Business address: | 29 Kirksway Place | | | | | |
| | Battery Point | 7004 | Fax No: | N/A | | |
| Licence No: | CC774A Email ac | ddress: office@geo | solutions.net.au | | | |
| Details of the p | roposed work: | | | | | |
| Owner/Applicant | Katie Ford | | Designer's projec | [↑] J11364 | | |
| [| Natie Fold | | reference No. | 311304 | | |
| Address: | 60 Treloggen Drive | | Lot No: | 104144/1 | | |
| | Binalong Bay | 7216 | | | | |
| Type of work: | Building wo | rk | Plumbing work | X (X all applicable) | | |
| Description of wor | | | | | | |
| On-Site stormwater | system - design | | , | w building / alteration / dition / repair / removal / | | |
| | | | | erection ater / sewerage / | | |
| | | | sto | rmwater / ·site wastewater | | |
| | | | ma | nagement system / | | |
| Description of the | Design Work (Scope, limitat | ions or exclusion | | ckflow prevention / other) | | |
| Certificate Type: | Certificate | | esponsible Prac | | | |
| , | ☐ Building design | | rchitect or Buildin | | | |
| | ☐ Structural design | E | ngineer or Civil D | Designer | | |
| | ☐ Fire Safety design | F | ire Engineer | | | |
| | | C | ivil Engineer or C | ivil Designer | | |
| | ☐ Hydraulic design | В | uilding Services [| Designer | | |
| | ☐ Fire service design | В | uilding Services [| Designer | | |
| | ☐ Electrical design | В | uilding Services [| Designer | | |
| | ☐ Mechanical design | В | uilding Service D | esigner | | |
| | ☐ Plumbing design Plumber-Certifier; Architect, Building Designer or Engineer | | | | | |
| | ☐ Other (specify) | · · · · · · · · · · · · · · · · · · · | <u> </u> | | | |
| Doomad to Satisfin | П | Performance Solu | ıtion: 🗶 💘 | | | |
| Deemed-to-Satisfy: | | renormance soil | JUOII. 🖭 (X the 8 | appropriate box) | | |
| Other details: | Other details: | | | | | |
| Onsite stormwater r | etention | | | | | |
| Design docume | Design documents provided: | | | | | |

The following documents are provided with this Certificate – Document description: Date: Jan-25 Drawing numbers: Prepared by: Geo-Environmental Solutions Prepared by: Schedules: Date: Specifications: Prepared by: Geo-Environmental Solutions Date: Jan-25 Computations: Prepared by: Date: Performance solution proposals: Prepared by: Geo-Environmental Solutions Date: Jan-25 Prepared by: Geo-Environmental Solutions Date: Jan-25 Test reports: Standards, codes or guidelines relied on in design process: AS3500 (Parts 0-5)-2013 Plumbing and drainage set. Any other relevant documentation: Stormwater Assessment - 60 Treloggen Drive Binalong Bay - Jan-25

Attribution as designer:

I Vinamra Gupta, am responsible for the design of that part of the work as described in this certificate;

The documentation relating to the design includes sufficient information for the assessment of the work in accordance with the *Building Act 2016* and sufficient detail for the builder or plumber to carry out the work in accordance with the documents and the Act;

This certificate confirms compliance and is evidence of suitability of this design with the requirements of the National Construction Code.

Designer:

Vinamra Gupta

Date

29/01/2025

Licence No: 685982720

| Assessment of Certifiable Works: (TasWater | Assessment | of Certifiable | Works: | (TasWater) |
|--|-------------------|----------------|--------|------------|
|--|-------------------|----------------|--------|------------|

Note: single residential dwellings and outbuildings on a lot with an existing sewer connection are not considered to increase demand and are not certifiable.

If you cannot check ALL of these boxes, LEAVE THIS SECTION BLANK.

TasWater must then be contacted to determine if the proposed works are Certifiable Works.

| rasvvater must the | in be contacted to determine if the | proposeu wo | iks are certifian | ie i | NOIKS. | | |
|---|--|-----------------|-------------------|------|----------------|--|--|
| | proposed works are not Certifiable beessments, by virtue that all of the | | | e G | Buidelines for | | |
| x The works wi | will not increase the demand for water supplied by TasWater | | | | | | |
| | ill not increase or decrease the amount of sewage or toxins that is to be removed by, d into, TasWater's sewerage infrastructure | | | | | | |
| | ll not require a new connection, or a n Vater's infrastructure | nodification to | an existing conne | ecti | on, to be | | |
| x The works wi | ll not damage or interfere with TasWa | ter's works | | | | | |
| x The works wi | ll not adversely affect TasWater's ope | erations | | | | | |
| x The work are | not within 2m of TasWater's infrastru | cture and are | outside any TasV | Vat | er easement | | |
| x I have checke | x I have checked the LISTMap to confirm the location of TasWater infrastructure | | | | | | |
| x If the property is connected to TasWater's water system, a water meter is in place, or has been applied for to TasWater. | | | | | | | |
| Certification: | | | | | | | |
| I Vinamra Gupta being responsible for the proposed work, am satisfied that the works described above are not Certifiable Works, as defined within the <i>Water and Sewerage Industry Act 2008</i> , that I have answered the above questions with all due diligence and have read and understood the Guidelines for TasWater CCW Assessments. Note: the Guidelines for TasWater Certification of Certifiable Works Assessments are available at: www.taswater.com.au | | | | | | | |
| | Name: (print) | | Signed | , , | Date | | |
| Designer: | Vinamra Gupta | Rupta | _ | | 29/01/2025 | | |

GEO-ENVIRONMENTAL ASSESSMENT

60 Treloggen Drive
Binalong Bay
January 2025







GEO-ENVIRONMENTAL

SOLUTIONS

Disclaimer: The author does not warrant the information contained in this document is free from errors or omissions. The author shall not in any way be liable for any loss, damage or injury suffered by the User consequent upon, or incidental to, the existence of errors in the information.



Investigation Details

Client: Katie Ford

Site Address: 60 Treloggen Drive, Binalong Bay

Date of Inspection: 28/01/2025

Proposed Works: New house

Investigation Method: Hand Auger

Inspected by: JP Cumming

Site Details

Certificate of Title (CT): 104144/1

Title Area: Approx. 928.6 m²

Applicable Planning Overlays: Bushfire-prone areas, Landslip Hazard, Priority Vegetation

Slope & Aspect: 7° SE facing slope

Vegetation: Mixed Flora

Background Information

Geology Map: MRT

Geological Unit: Devonian Granite

Climate: Annual rainfall 700mm

Water Connection: Tank

Sewer Connection: Unserviced-On-site required

Testing and Classification: AS2870:2011, AS1726:2017, AS1547:2012 & AS4055:2021



Investigation

A number of bore holes were completed to identify the distribution and variation of the soil materials at the site, bore hole locations are indicated on the site plan. See soil profile conditions presented below. Tests were conducted across the site to obtain bearing capacities of the material at the time of this investigation.

Soil Profile Summary

| BH 1 Depth (m) | BH 2 Depth (m) | USCS | Description |
|-------------------|-------------------|------|---|
| 0.0 – 0.40 | 0.0 – 0.30 | SW | Dark Grey SAND , slightly moist, medium dense consistency, trace of gravels, common fine rootlets, gradual boundary to |
| 0.40 - 0.90 | 0.30 – 0.80 | SW | Light Brownish Grey SAND , slightly moist to moist, medium dense consistency, trace of gravels, clear boundary to |
| 0.90 – 1.10 | 0.80 – 1.00 | SW | Black SAND , dry, dense consistency, ~10% quartz gravels, clear boundary to |
| 1.10 – 1.8+ | 1.00 – 1.5+ | SP | Light Brown SAND , dry, dense consistency, ~10% quartz gravels |

Site Notes

The soils on site consist of deep windblown sands which have developed over Devonian granite.

Site Classification

The site has been assessed and classified in accordance with AS2870:2011 "Residential Slabs and Footings".

The site has been classified as:

Class S

Y's range: 0-20mm

Notes: That is a slightly reactive site.



Wind Loading Classification

According to "AS4055:2021 - Wind Loads for Housing" the house site is classified below:

Wind Classification:N2Region:ATerrain Category:2.5Shielding Classification:NSTopographic Classification:T1Wind Classification:N2Design Wind Gust Speed - m/s (Vh,u):40

Wastewater Classification & Recommendations

According to AS1547-2012 (on-site waste-water management) the natural soil is classified as **SAND** (category 1). Secondary treatment of wastewater will be required due to the limited space available onsite. It is proposed to install a package treatment system (e.g. AWTS such as Econocycle, Envirocycle, Ozzikleen etc) with the treated wastewater applied into an absorption bed. A Design Loading Rate (DLR) of 40L/m²/day has therefore been applied for secondary treated wastewater.

The proposed two-bedroom dwelling has a calculated maximum wastewater output of 480L/day. This is based on a tank water supply and a maximum occupancy of 4 people (120L/day/person).

Using the DLR of $40L/m^2/day$, an absorption area of at least $12m^2$ will be required. This can be accommodated by one $10m \times 1.2m \times 0.6m$ absorption bed as per the attached design. The base of the absorption area will need to be ripped to ensure the soil beneath the bed is not compacted.

A cut off drain will be required upslope of the application area to divert any surface water flows. A 100% reserve area will also need to be set aside and keep free from development for any future wastewater requirements. There is sufficient space available onsite to accommodate the required reserve.

The following setback distances are required to comply with the Building Act 2016:

Upslope or level buildings: 3m

Downslope buildings: 3.75m

Upslope or level boundaries: 1.5m

Downslope boundaries: 8.5m

Downslope surface water: 29m

Compliance with Building Act 2016 Guidelines for On-site Wastewater Management Systems is outlined in the attached table.



Construction Notes & Recommendations

The site has been classified as **Class S** - Slightly reactive site, which may experience only slight ground movement from moisture changes.

It is recommended that all footings be founded in the natural material with bearing capacities >100kPa.

All earthworks on site must comply with AS3798:2012, and I further recommend that consideration be given to drainage and sediment control on site during and after construction. Care should also be taken to ensure there is adequate drainage in the construction area to avoid the potential for weak bearing and foundation settlement associated with excessive soil moisture.

During construction GES will need to be notified of any variation in the soil conditions or wastewater loading as outlined in this report.

Dr John Paul Cumming B.Agr.Sc (hons) PhD CPSS GAICD

Director







GES P/L

Land suitability and system sizing for on-site wastewater management

Trench 3.0 (Australian Institute of Environmental Health)

Assessment Report

Site assessment for on-site waste water disposal

Assessment for Katie Ford Assess. Date 29-Jan-25

Ref. No.

Assessed site(s) 60 Treloggen Drive Binalong Bay Site(s) inspected

Local authority Break O'Day Assessed by JP Cumming

This report summarises wastewater volumes, climatic inputs for the site, soil characteristics and sustem sizing and design issues. Site Capability and Environmental sensitivity issues are reported separately, where 'Alert' columns flag factors with high (A) or very high (AA) limitations which probably require special consideration for system design(s). Blank spaces on this page indicate data have not been entered into TRENCH.

Wastewater Characteristics

'astewater volume (L/day) used for this assessment = 480 (using the 'No. of bedrooms in a dwelling' method)

Septic tank wastewater volume (L/day) = 160

Sullage volume (L/day) = 320

Total nitrogen (kg/year) generated by wastewater = 1.5 otal phosphorus (kg/year) generated by wastewater = 1.2

Climatic assumptions for site

(Evapotranspiration calculated using the crop factor method)

| | Jan | Feb | Mar | Apr | May | Jun | Jul | Aug | Sep | Oct | Nov | Dec |
|---------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Mean rainfall (mm) | 51 | 46 | 56 | 55 | 46 | 49 | 68 | 67 | 70 | 68 | 64 | 62 |
| Adopted rainfall (R, mm) | 51 | 46 | 56 | 55 | 46 | 49 | 68 | 67 | 70 | 68 | 64 | 62 |
| Retained rain (Rr, mm) | 43 | 39 | 48 | 47 | 39 | 42 | 58 | 57 | 60 | 58 | 54 | 53 |
| Max. daily temp. (deg. C) | | | | | | | | | | | | |
| Evapotrans (ET, mm) | 130 | 110 | 91 | 63 | 42 | 29 | 32 | 42 | 63 | 84 | 105 | 126 |
| Evapotr. less rain (mm) | 87 | 71 | 43 | 16 | 3 | -12 | -26 | -15 | 3 | 26 | 51 | 73 |

Annual evapotranspiration less retained rain (mm) = 320

Soil characterisitics

Texture = Sand Category = 1 Thick. (m) = 2Adopted permeability (m/day) = 3 Adopted LTAR (L/sq m/day) = 40 Min depth (m) to water = 5

Proposed disposal and treatment methods

Proportion of wastewater to be retained on site: All wastewater will be disposed of on the site

The preferred method of on-site primary treatment: In a package treatment plant

The preferred method of on-site secondary treatment: In-ground

The preferred type of in-ground secondary treatment: Evapotranspiration bed(s)

The preferred type of above-ground secondary treatment: None

Site modifications or specific designs: Not needed

Suggested dimensions for on-site secondary treatment system

Total length (m) = 11

Width (m) = 1.2

Depth (m) = 0.6

Total disposal area (sq m) required = 12

comprising a Primary Area (sq m) of: 12

and a Secondary (backup) Area (sq m) of:

Sufficient area is available on site

To enter comments, click on the line below 'Comments'. (This yellow-shaded box and the buttons on this page will not be printed.)

Comments

Using the DLR of 40L/m2/day, an absorption area of 12m2 is required to accommodate the expected wastewater flows.







GES P/L

Land suitability and system sizing for on-site wastewater management

Trench 3.0 (Australian Institute of Environmental Health)

Site Capability Report Site assessment for on-site waste water disposal

Assessment for Katie Ford Assess. Date 29-Jan-25

Ref. No.

Assessed site(s) 60 Treloggen Drive Binalong Bay Site(s) inspected

Local authority Break O'Day Assessed by JP Cumming

This report summarises data relating to the physical capability of the assessed site(s) to accept wastewater. Environmental sensitivity and system design issues are reported separately. The 'Alert' column flags factors with high (A) or very high (AA) site limitations which probably require special consideration in site acceptability or for system design(s). Blank spaces indicate data have not been entered into TRENCH.

| | | | | Confid | | tation | |
|-------|-----------------------------|--------------|--------|---------|-----------|----------|---|
| Alert | Factor | Units | Value | level | Trench | Amended | Remarks |
| | Expected design area | sq m | 500 | V. high | High | Moderate | Other factors lessen impact |
| | Density of disposal systems | /sq km | 20 | Mod. | Moderate | | 000000000000000000000000000000000000000 |
| | Slope angle | degrees | 7 | High | Low | | 80000000 |
| | Slope form | Straight si | mple | High | Low | | |
| | Surface drainage | | Good | High | Very low | | 000000000000000000000000000000000000000 |
| | Flood potential Site | floods <1:10 | 00 yrs | High | Very low | | 000000000000000000000000000000000000000 |
| | Heavy rain events | Infred | quent | High | Moderate | | 200000000000000000000000000000000000000 |
| Α | Aspect (Southern hemi.) | Faces SE o | or SW | V. high | High | | |
| | Frequency of strong winds | Com | nmon | High | Low | | 000000000000000000000000000000000000000 |
| | Wastewater volume | L/day | 480 | High | Low | Moderate | 00000000 |
| | SAR of septic tank effluent | | 1.7 | High | Low | | |
| | SAR of sullage | | 2.6 | High | Moderate | | 000000000000000000000000000000000000000 |
| | Soil thickness | m | 2.0 | V. high | Very low | | |
| | Depth to bedrock | m | 3.0 | V. high | Very low | | 000000000000000000000000000000000000000 |
| | Surface rock outcrop | % | 0 | V. high | Very low | | 200000000 |
| | Cobbles in soil | % | 10 | V. high | Low | | |
| | Soil pH | | 5.5 | High | Low | | 0000000 |
| | Soil bulk density gn | n/cub. cm | 1.4 | High | Very low | | 000000000000000000000000000000000000000 |
| | Soil dispersion Eme | erson No. | 8 | V. high | Very low | | 000000000 |
| | Adopted permeability | m/day | 3 | Mod. | Very high | Moderate | Other factors lessen impact |
| | Long Term Accept. Rate L | /day/sq m | 40 | High | Very high | Moderate | Other factors lessen impact |

To enter comments, click on the line below 'Comments' . (This yellow-shaded box and the buttons on this page will not be printed.)

Comments

The site has the capability to accept secondary treated was tewater







GES P/L

Land suitability and system sizing for on-site wastewater management

Trench 3.0 (Australian Institute of Environmental Health)

Environmental Sensitivity Report Site assessment for on-site waste water disposal

Assessment for Katie Ford

Local authority Break O'Day

Assess. Date Ref. No. 29-Jan-25

Assessed site(s) 60 Treloggen Drive Binalong Bay

Site(s) inspected

Assessed by

JP Cumming

This report summarises data relating to the environmental sensitivity of the assessed site(s) in relation to applied wastewater. Physical capability and system design issues are reported separately. The 'Alert' column flags factors with high (A) or very high (AA) limitations which probably require special consideration in site acceptability or for system design(s). Blank spaces indicate data have not been entered into TRENCH.

| | | | | Confid | Limitation | | |
|-------|--------------------------------|----------|-------|---------|------------|------|---|
| Alert | Factor | Units | Value | level | Trench Ame | nded | Remarks |
| AA | Cation exchange capacity mm | ol/100g | 20 | High | Very high | | |
| Α | Phos. adsorp. capacity kg | g/cub m | 0.3 | High | High | | 000000000000000000000000000000000000000 |
| | Annual rainfall excess | mm | -320 | High | Very low | | 200000000000000000000000000000000000000 |
| | Min. depth to water table | m | 5 | High | Very low | | 000000000000000000000000000000000000000 |
| | Annual nutrient load | kg | 2.6 | High | Very low | | 800000000000000000000000000000000000000 |
| Α | G'water environ. value | Recreati | ional | V. high | High | | 000000000000000000000000000000000000000 |
| | Min. separation dist. required | m | 2 | High | Very low | | 200000000000000000000000000000000000000 |
| | Risk to adjacent bores | Ver | ylow | V. high | Very low | | 000000000000000000000000000000000000000 |
| Α | Surf. water env. value | Recreati | ional | V. high | High | | |
| AA | Dist. to nearest surface water | m | 45 | V. high | Very high | | 000000000000000000000000000000000000000 |
| | Dist. to nearest other feature | m | 20 | V. high | High | Low | Other factors lessen impact |
| | Risk of slope instability | | Low | V. high | Low | | |
| | Distance to landslip | m | 60 | V. high | Moderate | | 200000000000000000000000000000000000000 |

To enter comments, click on the line below 'Comments'. (This yellow-shaded box and the buttons on this page will not be printed.)

Comments

Secondary treatment of wastewater is required



Explanatory Notes

1 Scope of Works

The methods of description and classification of soils used in this report are based largely on Australian Standard 1726 – Geotechnical Site Investigations (AS1726:2017), with reference to Australian Standard 1289 – Methods for testing soils for engineering purposes (AS1289), for eventual Site Classification according to Australian Standard 2870 (AS2870:2011) – Residential Slabs and Footings and Australian Standard 1547 (AS1547:2012) On-site domestic wastewater management.

1.1 Site Classification AS2870:2011

Site classification with reference to the above Australian Standards are based on site reactivity.

| Class | Foundation Conditions | Characteristic Surface Movement |
|-------|--|------------------------------------|
| Α | Most sand and rock sites with little or no ground movement from moisture changes. | 0mm |
| S | Slightly reactive clay sites, which may experience only slight ground movement from moisture changes. | 0 – 20mm |
| М | Moderately reactive clay or silt sites, which may experience moderate ground movement from moisture changes. | 20 – 40mm |
| H-1 | Highly reactive clay sites, which may experience high ground movement from moisture changes. | 40 – 60mm |
| H-2 | Highly reactive clay sites, which may experience very high ground movement from moisture changes. | 60 – 75mm |
| Е | Extremely reactive sites, which may experience extreme ground movement from moisture changes. | >75mm |

Note: Soils where foundation performance may be significantly affected by factors other than reactive soil movement are classified as **Class P**.

A site is classified as Class P when:

- The bearing capacity of the soil profile in the foundation zone is generally less than 100kpa
- If excessive foundation settlement may occur due to loading on the foundation.
- The site contains uncontrolled fill greater than 0.8m in depth for sandy sites and 0.4m in depth for other soil materials.
- The site is subject to mine subsistence, landslip, collapse activity or coastal erosion.
- The site is underlain by highly dispersive soils with significant potential for erosion
- If the site is subject to abnormal moisture conditions which can affect foundation performance



1.2 Soil Characterisation

This information explains the terms of phrase used within the soil description area of the report.

It includes terminology for cohesive and non-cohesive soils and includes information on how the Unified Soil Classification Scheme (USCS) codes are determined.

| NON COHESIVE – SAND & GRAVEL | | | |
|------------------------------|---|--|--|
| Consistency Description | Field Test | Dynamic Cone Penetrometer blows/100 mm | |
| Very loose (VL) | Easily penetrated with 13 mm reinforcing rod pushed by hand. | 0 - 1 | |
| Loose (L) | Easily penetrated with 13 mm reinforcing rod pushed by hand. Can be excavated with a spade; 50 mm wooden peg can be easily driven. | 1 - 3 | |
| Medium dense (MD) | Penetrated 300 mm with 13 mm reinforcing rod driven with 2 kg hammer, - hard shovelling. | 3 - 8 | |
| Dense (D) | Penetrated 300 mm with 13 mm reinforcing rod driven with 2 kg hammer, requires pick for excavation: 50 mm wooden peg hard to drive. | 8 - 15 | |
| Very dense (VD) | Penetrated only 25 - 50 mm with 13 mm reinforcing rod driven with 2 kg hammer. | >15 | |

| COHESIVE - SILT & CLAY | | | | |
|-------------------------|--|---|--|--|
| Consistency Description | Field Test | Indicative undrained shear strength kPa | | |
| Very soft | Easily penetrated >40 mm by thumb. Exudes between thumb and fingers when squeezed in hand. | <12 | | |
| Soft | Easily penetrated 10 mm by thumb. Moulded by light finger pressure | >12 and <25 | | |
| Firm | Impression by thumb with moderate effort. Moulded by strong finger pressure | >25 and <50 | | |
| Stiff | Slight impression by thumb cannot be moulded with finger. | >50 and <100 | | |
| Very Stiff | Very tough. Readily indented by thumbnail. | >100 and <200 | | |
| Hard | Brittle. Indented with difficulty by thumbnail. | >200 | | |







1.3 USCS Material Descriptions

Soils for engineering purposes are the unconsolidated materials above bedrock, they can be residual, alluvial, colluvial or aeolian in origin.

| Major Divisions | | Particle size mm Symbol | | Typical Names | gi- | Laboratory Classification | | | | | |
|--|---|-------------------------|--|---|-----------------------------|---------------------------|-------------------------------|----------------------------------|---|---|--|
| | BOULDERS | 200 | | | % < | 0.075 mm (2) | Plasticity of fine fraction | $C_{ii} = \frac{D_{i0}}{D_{i0}}$ | $C_c = \frac{(D_{so})^2}{(D_{so})(D_{so})}$ | NOTES | |
| mm) | COBBLES | | | | | | | | | | |
| than 0.075 mm) | | 63 | GW | Well graded gravels and gravel-sand mixtures, little or no fines | | 0-5 | - | >4 | Between 1 and 3 | (1) Identify fines by the method give | |
| 98 | GRAVELS (more than | coarse | GP | Poorly graded gravels and gravel-sand mixtures, little or no fines, uniform gravels | Divisions | 0-5 | 4.00 | Fails to | comply with | for fine-grained soils. | |
| NED SC 63 mm | half of coarse | medium | GM | Silty gravels, gravel-sand-silt mixtures (1) | in 'Major | 12-50 | Below 'A' line or PI<4 | 1 555 | | | |
| | fraction is larger than 2.36 mm) | 6 fine 2.36 | GC | Clayey gravels, gravel-sand- clay mixtures (1) | gven | 12-50 | Above 'A' line and PI>7 | = | . = | (2) Borderline | |
| COARSE GRAI (more than half of material less than | SANDS (more than half of coarse fraction is smaller than 2.36 mm) | 3 | SW | Well graded sands and gravelly sands, little or no fines | the criteria | 0-5 | <u> 8228</u> | >6 | Between 1 and 3 | classifications occur when the percentage of fines (fraction | |
| | | of0.6 arse | SP | Poorly graded sands and gravelly sands, little or no fines | ording to | 0-5 | 8 | | comply with | smaller than 0.075 mm size is greater than 5% and less | |
| | | medium 0.2 | SM | Silty sands, sand silt mixtures (1) | INS BOC | 12-50 | Below 'A' line or PI<4 | = | 223 | than 12%. Borderline | |
| | | fine 0.075 | sc | Clayey sands, sand-clay mixtures (1) | n of fractic | 12-50 | Above 'A' line and PI>7 | _ | _ | classifications require the use of SP-SM, GW- GC. | |
| n 0.075 mm | SANDS (more than half of coarse half of coarse smaller than 2.36 mm) ME (more than half of coarse smaller than 2.36 mm) ME (more than half of coarse smaller than 2.36 mm) ME (more than half of coarse smaller than 2.36 mm) ME (more than half of coarse smaller than 2.36 mm) ME (more than 2.36 mm) Medium (mixtures (1)) SM (more than 2.36 mm) SP (poorly graded sands and grawelly sands, little or no fines smaller than 2.36 mm) ME (layey sands, sand-clay mixtures (1)) ME (more than 2.36 mm) SM (more than 2.36 mm) SM (silty sands, sand silt mixtures (1)) ME (layey sands, sand-clay mixtures (1)) ME (layey sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity ME (liquid Limit \$50%) CL (layey sands or clayey silts with slight plasticity) ME (liquid Limit \$50%) CL (layey sands or clayey silts with slight plasticity) ME (liquid Limit \$50%) CL (layey sands sand silt mixtures (1)) ME (layey sands sand sand sand sand sand sand san | | sands, rock flour, silty or clayey fine sands or clayey | Plasticity Chart For classification of fine grained soils and fine fraction of coarse grained soils. | | | | ined soils | | | |
| smaller | | | | | dum High | alned soils. | | | | | |
| SOILS mm is | | | OL | Organic silts and clays of low plasticity | bassed | | | | | 41.70 | |
| FINE GRAINED SOILS ifal less than 63 mm is | | МН | | Inorganic silts, mic- aceous or diato-maceous fine sands or silts, elastic silts | gradation curve of material | Plastic Index (%) | | | 4 | Trips Traffill 2 | |
| FINE (more than half of material les | SILTS & CLA (Liquid Limit | | CH | Inorganic clays of high plasticity, fat clays | curve | 320.53 | 15/85 | - C | O MH&C | 91 | |
| | | | ОН | Organic silts and clays of high plasticity | adation | 10 | 100 | MI | 801 | | |
| | HIGHLY OR | SANIC | PT | Peat and other highly organic soils | Use the gr | 0 | 10 20 | 30 40 Liqu | s 60 uid Limit (%) | 70 80 90 10 | |



Grain size analysis is performed by two processes depending on particle size. Sand silt and clay particles are assessed using a standardised hydrometer test, and coarse sand and larger is assessed through sieving by USCS certified sieves. For more detail see the following section.

| Soil Classification | Particle Size |
|---------------------|-------------------|
| Clay | Less than 0.002mm |
| Silt | 0.002 – 0.06mm |
| Fine/Medium Sand | 0.06 – 2.0mm |
| Coarse Sand | 2.0mm – 4.75mm |
| Gravel | 4.75mm – 60.00mm |

1.4 Bearing Capacities and DCP testing.

DCP and PSP weighted penetrometer tests – Dynamic Cone Penetrometer (DCP) and Perth Sand Penetrometer (PSP) tests are carried out by driving a rod into the ground with a falling weight hammer and measuring the blows for successive 100mm increments of penetration. Normally, there is a depth limitation of 1.2m but this may be extended in certain conditions by the use of extension rods. The methods for the two tests are quite similar.

- Dynamic Cone Penetrometer a 16mm rod with a 20mm diameter cone end is driven with a 9kg hammer dropping 510mm (AS 1289, Test 6.3.2).
- Perth Sand Penetrometer a 16mm diameter flat-ended rod is driven with a 9kg hammer, dropping 600mm (AS 1289 Test 6.3.3). This test was developed for testing the density of sands and is mainly used in granular soils and filling.

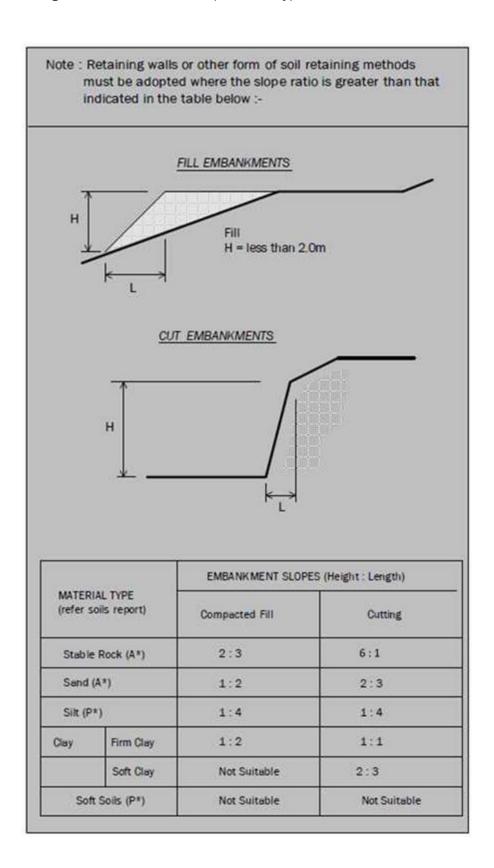
Site Anomalies – During construction GES will need to be notified of any major variation to the foundation conditions as predicted in this report.







1.5 Batter Angles for Embankments (Guide Only)





Glossary of Terms

Bearing Capacity – Maximum bearing pressure that can be sustained by the foundation from the proposed footing system under service loads which should avoid failure or excessive settlement.

Clay – (Mineral particles less than 0.002mm in diameter). Fine grained cohesive soil with plastic properties when wet. Also includes sandy clays, silty clays, and gravelly clays.

Dynamic Cone Penetrometer (DCP) – Field equipment used to determine underlying soil strength and therefore bearing capacity (kPa) by measuring the penetration of the device into the soil after each hammer blow.

Dispersive soil – A soil that has the ability to pass rapidly into suspension in water.

Footing – Construction which transfers the load from the building to the foundation.

Foundation – Ground which supports the building

Landslip – Foundation condition on a sloping site where downhill foundation movement or failure is a design consideration.

Qualified Engineer – A professional engineer with academic qualifications in geotechnical or structural engineering who also has extensive experience in the design of the footing systems for houses or similar structures.

Reactive Site – Site consisting of clay soil which swells on wetting and shrinks on drying by an amount that can damage buildings on light strip footings or unstiffened slabs. Includes sites classified as S, M, H-1, H-2 & E in accordance with AS2870-2011.

Sand – (Mineral particles greater than 0.02mm in diameter). Granular non-cohesive, non-plastic soil that may contain fines including silt or clay up to 15%.

Services – Means all underground services to the site including but not limited to power, telephone, sewerage, water & storm water.

Silt – (Mineral particles 0.002 – 0.02mm in diameter). Fine grained non-cohesive soil, non-plastic when wet. Often confers a silky smoothness of field texture, regularly includes clay and sand to form clayey silts, sandy silts and gravelly silts.

Site – The site title, as denoted by address, lot number, or Certificate of Title (CT) number, or Property Identification Number (PID).

Surface Movement (Ys) – Design movement (mm) at the surface of a reactive site caused by moisture changes.



Disclaimer

This Report has been prepared in accordance with the scope of services between Geo-Environmental Solutions Pty. Ltd. (GES) and the Client. To the best of GES's knowledge, the information presented herein represents the client's requirements at the time of printing of the Report. However, the passage of time, manifestation of latent conditions or impacts of future events may result in findings differing from that discussed in this Report. In preparing this Report, GES has relied upon data, surveys, analyses, designs, plans and other information provided by the Client and other individuals and organisations referenced herein. Except as otherwise stated in this Report, GES has not verified the accuracy or completeness of such data, surveys, analyses, designs, plans and other information.

The scope of this study does not allow for the review of every possible geotechnical parameter or the soil conditions over the whole area of the site. Soil and rock samples collected from the investigation area are assumed to be representative of the areas from where they were collected and not indicative of the entire site. The conclusions discussed within this report are based on observations and/or testing at these investigation points.

This report does not purport to provide legal advice. Readers of the report should engage professional legal practitioners for this purpose as required.

No responsibility is accepted for use of any part of this report in any other context or for any other purpose by a third party.

| Acceptable Solutions | Performance Criteria | Compliance |
|---|---|--|
| A1 Horizontal separation distance from a building to a land application area must comply with one of the following: a) be no less than 6m; or b) be no less than: (i) 3m from an upslope building or level building; (ii) If primary treated effluent to be no less than 4m plus 1m for every degree of average gradient from a downslope building; (iii) If secondary treated effluent and subsurface application, no less than 2m plus 0.25m for every degree of average gradient from a downslope building. | a) The land application area is located so that (i) the risk of wastewater reducing the bearing capacity of a building's foundations is acceptably low.; and (ii) is setback a sufficient distance from a downslope excavation around or under a building to prevent inadequately treated wastewater seeping out of that excavation | Complies with A1 (b) (i) Land application area will be located with a minimum separation distance of 3m from an upslope or level building. |
| Horizontal separation distance from downslope surface water to a land application area must comply with (a) or (b) (a) be no less than 100m; or (b) be no less than the following: (i) if primary treated effluent 15m plus 7m for every degree of average gradient to downslope surface water; or (ii) if secondary treated effluent and subsurface application, 15m plus 2m for every degree of average gradient to down slope surface water. | P2 Horizontal separation distance from downslope surface water to a land application area must comply with all of the following: a) Setbacks must be consistent with AS/NZS 1547 Appendix R; b) A risk assessment in accordance with Appendix A of AS/NZS 1547 has been completed that demonstrates that the risk is acceptable. | Complies with A2 (b) (ii) Land application area will be located with a minimum separation distance of 29m of downslope surface water |

| A3 | P3 | |
|--|---|--|
| Horizontal separation distance from a property boundary to a land application area must comply with either of the following: (a) be no less than 40m from a property boundary; or (b) be no less than: (i) 1.5m from an upslope or level property boundary; and (ii) If primary treated effluent 2m for every degree of average gradient from a downslope property boundary; or (iii) If secondary treated effluent and subsurface application, 1.5m plus 1m for every degree of average gradient from a downslope property boundary. | Horizontal separation distance from a property boundary to a land application area must comply with all of the following: (a) Setback must be consistent with AS/NZS 1547 Appendix R; and (b) A risk assessment in accordance with Appendix A of AS/NZS 1547 has been completed that demonstrates that the risk is acceptable. | Complies with A3 (b) (i) Land application area will be located with a minimum separation distance of 1.5m from an upslope or level property boundary Complies with A3 (b) (iii) Land application area will be located with a minimum separation distance of 8.5m of downslope property boundary |
| A4 Horizontal separation distance from a downslope bore, well or similar water supply to a land application area must be no less than 50m and not be within the zone of influence of the bore whether up or down gradient. | P4 Horizontal separation distance from a downslope bore, well or similar water supply to a land application area must comply with all of the following: (a) Setback must be consistent with AS/NZS 1547 Appendix R; and (b) A risk assessment completed in accordance with Appendix A of AS/NZS 1547 demonstrates that the risk is acceptable | Complies with A4 No bore or well identified within 50m |

| Vertical separation distance between groundwater and a land application area must be no less than: (a) 1.5m if primary treated effluent; or (b) 0.6m if secondary treated effluent | P5 Vertical separation distance between groundwater and a land application area must comply with the following: (a) Setback must be consistent with AS/NZS 1547 Appendix R; and (b) A risk assessment completed in accordance with Appendix A of AS/NZS 1547 that demonstrates that the risk is acceptable | Complies with A5 (b) |
|--|--|----------------------|
| A6 Vertical separation distance between a limiting layer and a land application area must be no less than: (a) 1.5m if primary treated effluent; or (b) 0.5m if secondary treated effluent | P6 Vertical setback must be consistent with AS/NZS1547 Appendix R. | Complies with A6 (b) |
| A7 nil | P7 A wastewater treatment unit must be located a sufficient distance from buildings or neighbouring properties so that emissions (odour, noise or aerosols) from the unit do not create an environmental nuisance to the residents of those properties | Complies |



AS1547:2012 – Loading Certificate – AWTS Design

This loading certificate sets out the design criteria and the limitations associated with use of the system.

Site Address: 60 Treloggen Drive, Binalong Bay

System Capacity: 4 persons @ 120L/person/day

Summary of Design Criteria

DLR: $40L/m^2/day$.

Absorption area: 12m²

Reserve area location /use: Assigned - more than 100% available

Water saving features fitted: Standard fixtures

Allowable variation from design flows: 1 event @ 200% daily loading per quarter

Typical loading change consequences: Expected to be minimal due to use of AWTS and large land area

Overloading consequences: Continued overloading may cause hydraulic failure of the absorption area and require upgrading/extension of the area. Risk considered acceptable due to monitoring through quarterly maintenance reports.

Underloading consequences: Lower than expected flows will have minimal consequences on system operation unless the house has long periods of non occupation. Under such circumstances additional maintenance of the system may be required. Long term under loading of the system may also result in vegetation die off in the absorption area and additional watering may be required. Risk considered acceptable due to monitoring through quarterly maintenance reports.

Lack of maintenance / monitoring consequences: Issues of underloading/overloading and condition of the irrigation area require monitoring and maintenance, if not completed system failure may result in unacceptable health and environmental risks. Monitoring and regulation by the permit authority required to ensure compliance.

Other considerations: Owners/occupiers must be made aware of the operational requirements and limitations of the system by the installer/maintenance contractor.

CERTIFICATE OF QUALIFIED PERSON – ASSESSABLE ITEM

Section 321

| To: | Katie Ford | Owner /Agent | | | |
|---|--|--|--|--|--|
| | 10 Hilly Street | Address Form 55 | | | |
| | Mortlake 2137 | Suburb/postcode | | | |
| Qualified perso | on details: | | | | |
| Qualified person: | John-Paul Cumming | | | | |
| Address: | 29 Kirksway Place | Phone No: 03 6223 1839 | | | |
| | Battery Point 7004 | Fax No: | | | |
| Licence No: | | ing@geosolutions.net.au | | | |
| Qualifications and Insurance details: | Scientist (CPSS stage 2) | escription from Column 3 of the rector's Determination - Certificates Qualified Persons for Assessable ms | | | |
| Speciality area of expertise: | AS2870-2011 Foundation Classification (description from Column 4 of the Director's Determination - Certificates by Qualified Persons for Assessable Items) | | | | |
| Details of work | (; | | | | |
| Address: | 60 Treloggen Drive | Lot No: | | | |
| | Binalong Bay 7216 | Certificate of title No: 104144/1 | | | |
| The assessable item related to this certificate: | Classification of foundation Conditions according to AS2870-2011 (description of the assessable item being certified) Assessable item includes – - a material; - a design - a form of construction - a document - testing of a component, building system or plumbing system - an inspection, or assessment, performed | | | | |
| Certificate deta | ails: | | | | |
| Certificate type: Foundation Classification (description from Column 1 of Schedule 1 of the Director's Determination - Certificates by Qualified Persons for Assessable Items n) | | | | | |
| This certificate is in | n relation to the above assessable item, at any st | age, as part of - (tick one) | | | |
| | building work, plumbing work or plumbing or | installation or demolition work | | | |
| | | ructure or plumbing installation: | | | |

In issuing this certificate the following matters are relevant –

Documents: The attached soil report for the address detailed above in 'details of

work'

Relevant

calculations: Reference the above report.

References: AS2870:2011 residential slabs and footings

AS1726:2017 Geotechnical site investigations

CSIRO Building technology file - 18.

Substance of Certificate: (what it is that is being certified)

Site Classification consistent with AS2870-2011.

Scope and/or Limitations

The classification applies to the site as inspected and does not account for future alteration to foundation conditions as a result of earth works, drainage condition changes or variations in site maintenance.

I, John-Paul Cumming certify the matters described in this certificate.

Qualified person:

Signed:

Certificate No:

Date:

J11364

29/01/2025



CERTIFICATE OF THE RESPONSIBLE DESIGNER

Section 94 Section 106 Section 129 Section 155

| To: | Katie Ford | | | | Owner name | 25 | |
|---|--|---------|------------|---------|--------------------------------------|--|--|
| | 10 Hilly Street | | | Address | Form 35 | | |
| | Mortlake | | 2137 | 7 | Suburb/postcode | | |
| Decimon detail | | | | | | | |
| Designer detail | S : | | | | | | |
| Name: | John-Paul Cumming | | | | Category: | Bld. Srvcs. Dsgnr Hydraulic | |
| Business name: | Geo-Environmental Solutions | 3 | | | Phone No: | 03 6223 1839 | |
| Business address: | 29 Kirksway Place | | | | | | |
| | Battery Point | | 7004 | | Fax No: | N/A | |
| Licence No: | CC774A Email ac | ddress: | office@g | jeoso | olutions.net.au | | |
| Details of the p | roposed work: | | | | | | |
| | | | | | Designer's project | cf 1440C4 | |
| Owner/Applicant | Katie Ford | | | | reference No. | ^{-t} J11364 | |
| Address: | 60 Treloggen Drive | | | | Lot No: | 104144/1 | |
| | Binalong Bay | | 7216 | 3 | | | |
| Type of work: | Building wo | rk 🗌 | | F | Plumbing work | X (X all applicable) | |
| Description of wor | rk: management system - design | | | | (10.0 | ew building / alteration / | |
| Description of the | Design Work (Scope, limitat | tions o | r exclusio | ons) | wa sto on- ma bad | erection hter / sewerage / rmwater / -site wastewater hagement system / ckflow prevention / other) certificates) | |
| Certificate Type: | Certificate | | | | sponsible Prac | | |
| , | ☐ Building design | | | | hitect or Buildin | | |
| | ☐ Structural design | | | Eng | gineer or Civil D | esigner | |
| | ☐ Fire Safety design | | | Fire | ire Engineer | | |
| | ☐ Civil design | | | Civi | il Engineer or C | Civil Designer | |
| | | | | Buil | Iding Services I | Designer | |
| | ☐ Fire service design | | | Buil | Iding Services I | Designer | |
| | ☐ Electrical design | | | | Iding Services I | | |
| | ☐ Mechanical design | | | | Iding Service D | | |
| | ☐ Plumbing design | | | | mber-Certifier; . signer or Engin | Architect, Building eer | |
| | ☐ Other (specify) | | | | 3 | | |
| Deemed-to-Satisfy: | | | rmance S | olutio | on: X the a | appropriate box) | |
| Other details: | Other details: | | | | | | |
| AWTS with absorption bed | | | | | | | |
| Design documents provided: | | | | | | | |

The following documents are provided with this Certificate – Document description: Drawing numbers: Date: Jan-25 Prepared by: Geo-Environmental Solutions Schedules: Prepared by: Date: Specifications: Prepared by: Geo-Environmental Solutions Date: Jan-25 Computations: Prepared by: Date: Performance solution proposals: Prepared by: Date: Prepared by: Geo-Environmental Solutions Test reports: Date: Jan-25 Standards, codes or guidelines relied on in design process: AS1547:2012 On-site domestic wastewater management. AS3500 (Parts 0-5)-2013 Plumbing and drainage set. Any other relevant documentation: Geo-Environmental Assessment - 60 Treloggen Drive Binalong Bay - Jan-25

Geo-Environmental Assessment - 60 Treloggen Drive Binalong Bay - Jan-25

Attribution as designer:

I John-Paul Cumming, am responsible for the design of that part of the work as described in this certificate;

The documentation relating to the design includes sufficient information for the assessment of the work in accordance with the Building Act 2016 and sufficient detail for the builder or plumber to carry out the work in accordance with the documents and the Act;

This certificate confirms compliance and is evidence of suitability of this design with the requirements of the National Construction Code.

| | Name: (print) | Signed | Date |
|-------------|-------------------|--------|------------|
| Designer: | John-Paul Cumming | | 29/01/2025 |
| Licence No: | CC774A | | |

Assessment of Certifiable Works: (TasWater)

Note: single residential dwellings and outbuildings on a lot with an existing sewer connection are not considered to increase demand and are not certifiable.

If you cannot check ALL of these boxes, LEAVE THIS SECTION BLANK.

TasWater must then be contacted to determine if the proposed works are Certifiable Works.

I confirm that the proposed works are not Certifiable Works, in accordance with the Guidelines for TasWater CCW Assessments, by virtue that all of the following are satisfied:

| | The works will not increase the demand for water supplied by TasWater |
|---|--|
| Х | The works will not increase the demand for water supplied by Taswater |
| Х | The works will not increase or decrease the amount of sewage or toxins that is to be removed by or discharged into, TasWater's sewerage infrastructure |
| Х | The works will not require a new connection, or a modification to an existing connection, to be made to TasWater's infrastructure |
| Х | The works will not damage or interfere with TasWater's works |
| Х | The works will not adversely affect TasWater's operations |
| Х | The work are not within 2m of TasWater's infrastructure and are outside any TasWater easemen |
| Х | I have checked the LISTMap to confirm the location of TasWater infrastructure |

| | applied for to TasWater. | l asvvater's water system, a water meter is in place, or has beel |
|--|--------------------------|---|
| | _ | |

Certification:

I John-Paul Cumming....... being responsible for the proposed work, am satisfied that the works described above are not Certifiable Works, as defined within the *Water and Sewerage Industry Act 2008*, that I have answered the above questions with all due diligence and have read and understood the Guidelines for TasWater CCW Assessments.

Note: the Guidelines for TasWater Certification of Certifiable Works Assessments are available at: www.taswater.com.au

Designer:

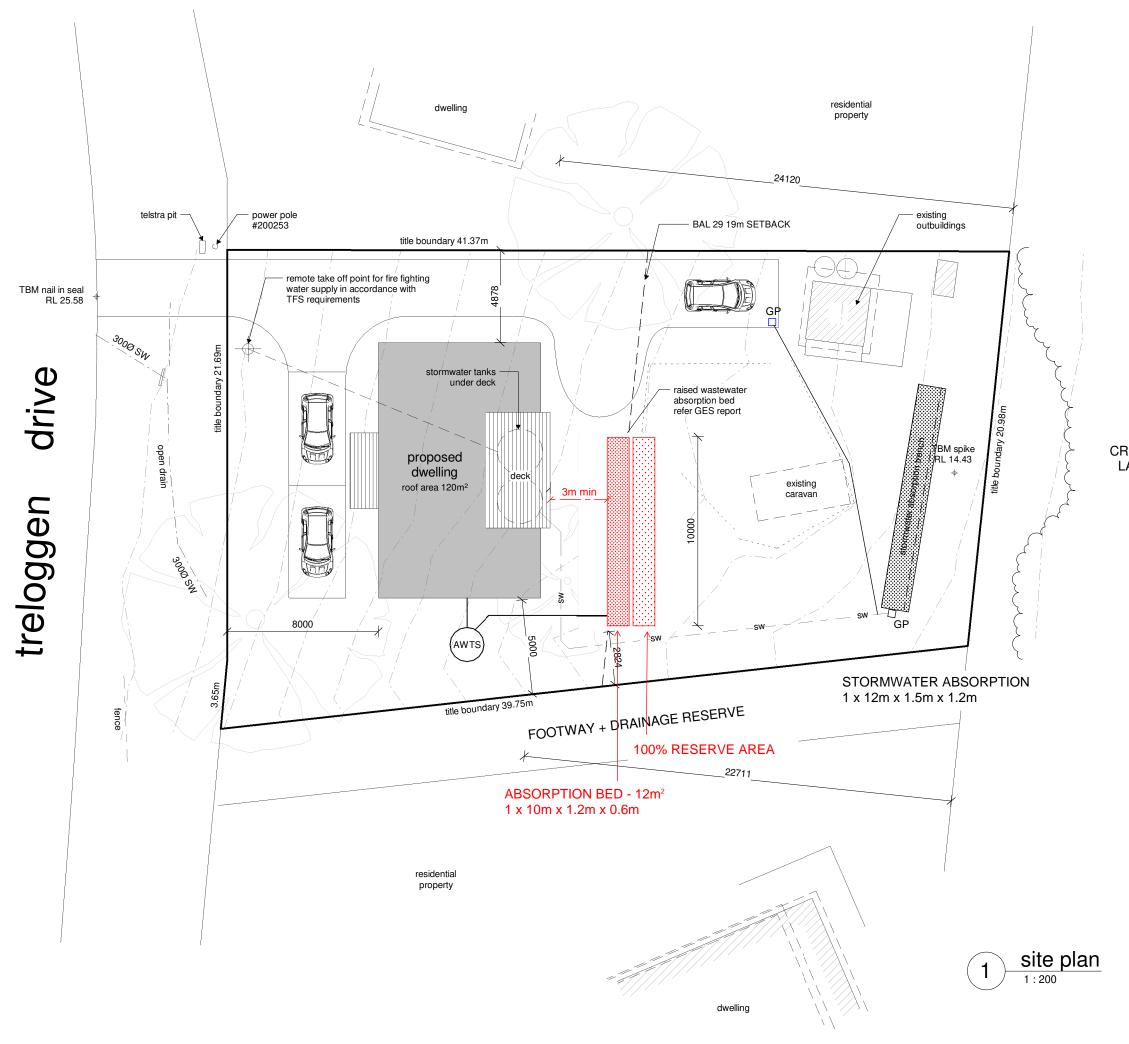
John-Paul Cumming

Name: (print)

Signed

Date 29/01/2025





Wastewater system:

AWTS unit located to ensure min 1:60 fall from all fixtures. Vented according to NCC Vol 3 Tas C2D6

Cut-off drain

Absorption Bed 3 x 10m x 1.2m x 0.6m

Min 3m from upslope buildings
Min 1.5m from upslope or level boundaries
Min 8.5m from downslope boundary
Min 29m from downslope surface water

Refer to GES report

Dr. John Paul Cumming Building Services Designer-Hydraulic CCC774A



29/01/2025

CROWN LAND



| 1 | Revision 1 | Date 1 | |
|-----------------------|-------------------|--------|--|
| REV: | DESCRIPTION: | DATE: | |
| PROJECT: | | | |
| pro | proposed dwelling | | |
| FOR: | | | |
| k. ford | | | |
| 60 treloggen drive | | | |
| binalong bay TAS 7216 | | | |

DRAWING TITLE:

site plan

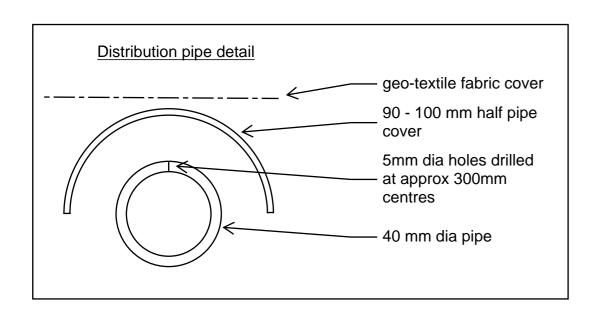
| DRAWING NO: | DRAWN BY: JB | | |
|--------------|-----------------|--|--|
| a04 | DATE: 23.01.25 | | |
| SCALE: 1:200 | PROJECT: 1123FO | | |



www.jenniferbinnsdesign.com.au 0439 765 452 : mail @ jenniferbinnsdesign.com.au 52 cecilia street st helens tasmania 7216

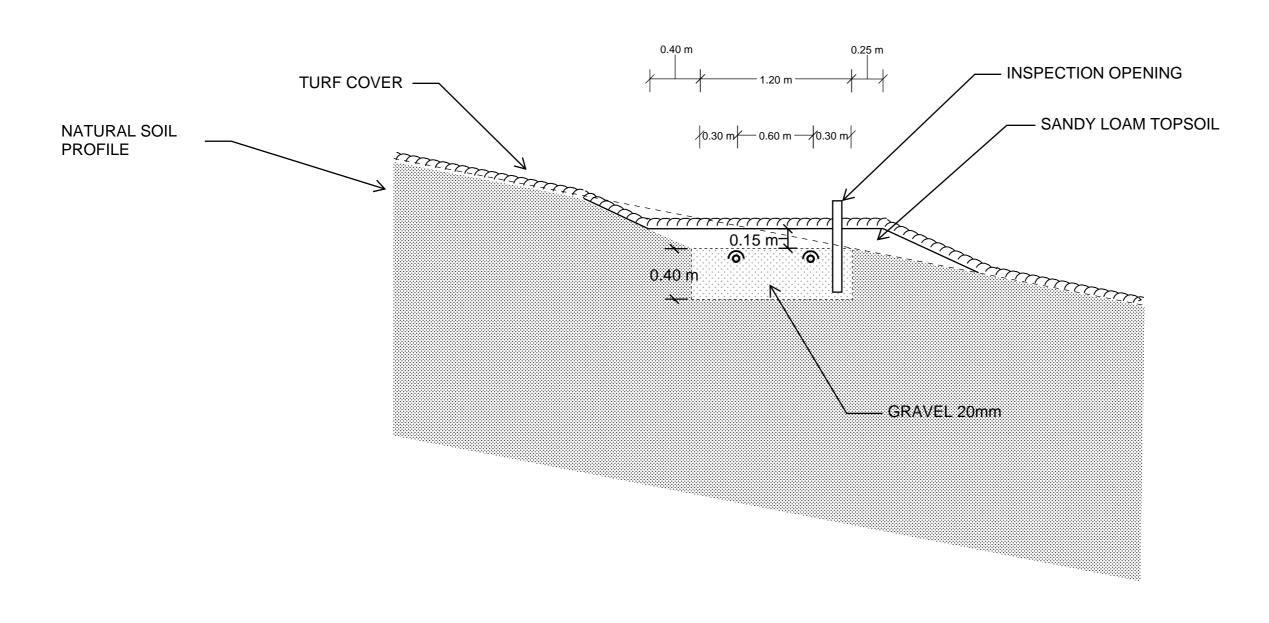


ACCREDITATION NO: CC 1269L





29 Kirksway Place Battery Point T| 62231839 E| office@geosolutions.net.au



Do not scale from these drawings. Dimensions to take precedence over scale.

Date: August 2018

On-site Wastewater Cross-Section

Sheet 1 of 2

Design notes:

- 1. Absorption bed dimensions of up to 15m long by 0.60m deep by 1.2m wide.
- 2. Base of bed to be excavated level min 200mm into natural soils and smearing and compaction avoided.
- 3. Lower 450mm of bed to be filled with 20mm clean washed gravel and drilled 40mm distribution pipes packed into upper 100mm of sand.
- 4. 40mm distribution pipes drilled with sufficient 5mm holes in the top of the pipe (approx spacing 300mm) to distribute the effluent and half circle 90-100mm UPVC pipe, un-perforated, laid over each 40mm perforated lateral to direct water jet downwards.
- 5. One 5 mm hole at centre of invert of each pipe to allow for drainage between pump cycles.
- 6. Geotextile or filter cloth to be placed over the distribution pipes to prevent clogging of the pipes and aggregate the sides of the bed should also be lined.
- 7. Final finished surface with sandy loam to be a minimum of 150 mm above aggregate with turf cover or mulched with appropriate vegetation (eg native grasses and small shrubs at 1 plant per 1 m2)
- 8. The turf or vegetation is an essential component of the system and must be maintained with regular moving and or trimming as appropriate
- 9. The distribution pipe grid must be absolutely level to allow even distribution of effluent around the absorption area it is recommended that the level be verified by running water into the system before backfilling and commissioning the trench
- 10. All works on site to comply with AS3500 and Tasmanian Plumbing code.

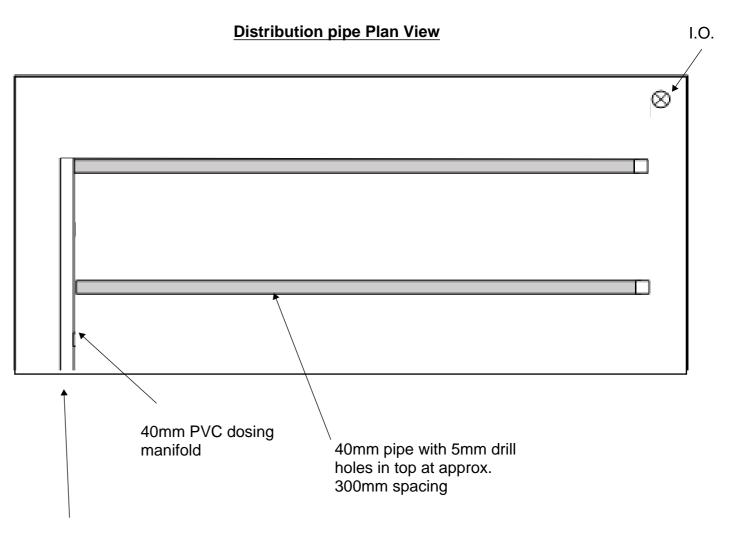
The pump must be capable of delivering the total flow rate required for all laterals whilst providing a 1.5m residual head (ie squirt height) at the highest orifice (with no more than 15% variation in squirt height across the whole bed).

For beds with individual laterals, no more than 15m long, it is acceptable to adopt a flow rate of 4-5L/min/lineal metre. Total dynamic head (including friction loss) will need to be determined on a site-specific basis.

Individual flush points must be installed for each lateral. This may be a screw cap fitting on a 90 degree elbow level with the bed surface or a pressure controlled flush valve inside an irrigation control box.



29 Kirksway Place Battery Point T| 62231839 E| office@geosolutions.net.au



Pump dosed effluent from treatment system.

Do not scale from these drawings.
Dimensions to take precedence over scale.

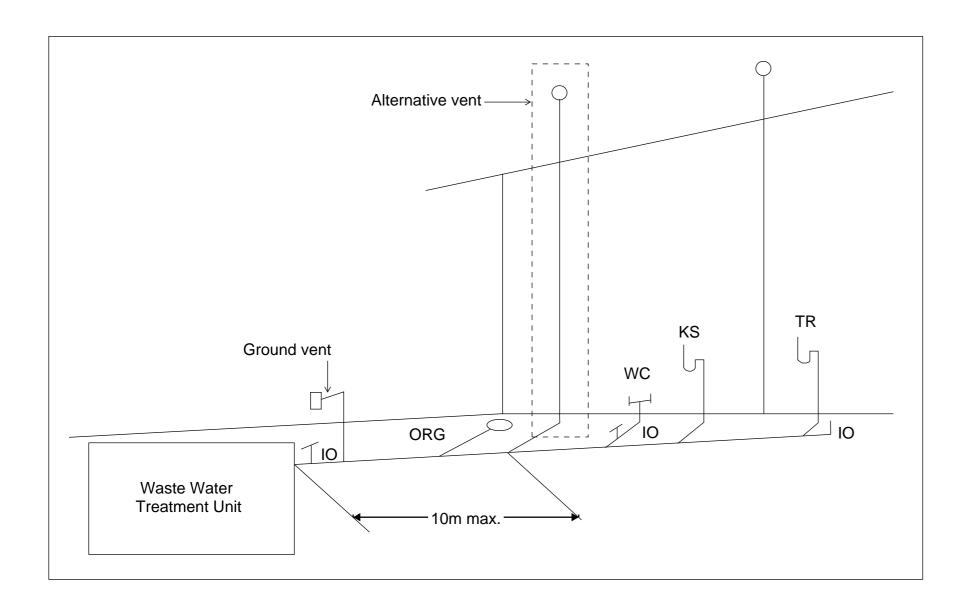
Date: April 2018

On-site Wastewater Design Notes

Sheet 2 of 2



29 Kirksway Place, Battery Point T| 62231839 E| office@geosolutions.net.au



Tas Figure C2D6 Alternative Venting Arrangements

Vents must terminate in accordance with AS/NZS 3500.2

Alternative venting to be used by extending a vent to terminate as if an upstream vent, with the vent connection between the last sanitary fixture or sanitary appliance and the on-site wastewater management system. Use of a ground vent in not recommended

Inspection openings must be located at the inlet to an on-site wastewater management system treatment unit and the point of connection to the land application system and must terminate as close as practicable to the underside of an approved inspection opening cover installed at the finished surface level

Access openings providing access for desludging or maintenance of on-site wastewater management system treatment unites must terminate at or above finished surface level

| Do not scale from these drawings. | | |
|-----------------------------------|--|--|
| Dimensions to take precedence | | |
| over scale. | | |